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# Stormwater Management Report

Proposed Industrial Redevelopment

Block 70.01, Lots 1.01, 1.02, 4.02 & 4.03 Borough of Wallington Bergen County, New Jersey

## September 4, 2019

Prepared For

Umdasch Real Estate USA, Ltd. 214 Gates Road Little Ferry, NJ 07643

Prepared By

Maser Consulting P.A. 50 Chestnut Ridge Road, Suite 101 Montvale NJ, 07645 845-352-0411





Engineers Planners Surveyors Landscape Architects Environmental Scientists

#### Stormwater Management Report – Index Page

## Project Name: Proposed Industrial Redevelopment Project Location: Borough of Wallington, Bergen County, New Jersey

The following table summarizes typical additional information that can be found throughout this Stormwater Management Report, Appendices and Supplemental Reports as indicated.

Description	Information	Location of Information	
Total Amount of Land Disturbed on Site	±20.04 Acres	Page # 3	
Acreage of Total Impervious Surfaces	±21.42 Acres Full Buildout	Page # 3	
Type of Basin Proposed	N/A	N/A	
Runoff Quantity Reqs & Attenuation	2-, 10- and 100-Years Storms	Page # 4	
Water Quality Reqs & Method	N/A – Less than 0.25 acres new impervious	Page # 4	
Groundwater Recharge Reqs & Method	N/A – Impacted soil	Page #5	
Runoff Quantity Calculations	HydroCAD Output	Appendix # 01	
Stormwater Conveyance System	25-year storm	Appendix # 02	
Groundwater Recharge Calculations	N/A	N/A	
Water Quality Calculations	N/A	N/A	
USGS, Soil Survey Location Map	Online Mapping	Appendix # 03	
Existing, Proposed and Inlet Area Map	Watershed Maps	Appendix # 04	
Operations and Maintenance Manual	Operations and Maintenance Manual	(Separate Document)	



**LIMITATION:** The sole purpose of this report and the associated services performed by Maser Consulting P.A. (Maser Consulting) is to undertake a hydrologic assessment for the proposed warehouse and office development in accordance with the scope of services set out in the contract between Maser Consulting and Umdasch Real Estate USA, Ltd.

Maser Consulting derived the data in this report from a variety of sources. The sources are identified at the time or times outlined in this report. The passage of time, manifestation of latent conditions or impacts of future events may require further examination of the project and subsequent data analysis, and re-evaluation of the data, findings, observations and conclusions expressed in this report. Maser Consulting has prepared this report in accordance with the usual care and thoroughness of the consulting profession, for the sole purpose of the project and by reference to applicable standards, procedures and practices at the date of issue of this report. For the reasons outlined above, however, no other warranty or guarantee, whether expressed or implied, is made as to the data, observations and findings expressed in this report.

This report should be read in full and no excerpts are to be taken as representative of the findings. No responsibility is accepted by Maser Consulting for use of any part of this report in any other context.

This report has been prepared on behalf of, and for the exclusive use of Umdasch Real Estate USA, Ltd., and is subject to, and issued in connection with, the provisions of the agreement between Maser Consulting and Umdasch Real Estate USA, Ltd.



#### **INTRODUCTION:**

This Stormwater Management Report for the proposed warehouse and office development located at 520 Main Avenue in Wallington, New Jersey, has been prepared by Maser Consulting for Umdasch Real Estate USA, Ltd. Maser Consulting performed a hydrological assessment for the aforementioned property, also defined as Block 70.01, Lots 1.01, 1.02, 4.02 & 4.03 on the Borough of Wallington Tax Maps. The site currently is 26.10 acres in size and is comprised of several industrial buildings with appurtenant site improvements throughout and is primarily paved. Under the current application, the applicant proposes to construct three new buildings for warehousing and office with typical appurtenant site improvements, which will result in a total disturbance of 20.04 acres. It is important to note that the impervious/gravel coverage on site will be reduced in the proposed condition as compared to the existing condition. Below is a breakdown of the impervious/gravel and pervious coverages on site for the existing and proposed conditions.

Coverage Type	Existing Conditions	<b>Proposed Conditions</b>	Difference
Impervious/Gravel	22.42 ac	21.42 ac	-1.00 ac
Pervious	3.68 ac	4.68 ac	+1.00 ac
Total	26.1 ac	26.1 ac	-

#### **PURPOSE:**

This report assesses the hydrologic aspects and the stormwater drainage conditions as a result of the proposed warehouse and office development, including building, parking and streets, landscaping areas, stormwater management facilities, associated utilities, and related site improvements.

The focus of the study is the identification of the existing drainage characteristics in comparison to post-development conditions. Calculations documenting the design of the stormwater management system are illustrated on the Site Plan drawing documents, prepared by Maser Consulting.

#### METHOD:

Generally, the majority of the stormwater runoff from the site is collected by inlets throughout the site and piped to the storm conveyance system in Main Avenue. Additionally, there is a small portion of the site at the northern end that discharges via overland flow to Main Avenue and flows down the steep road to the northeast. There is also a ridge line in the southern portion of the site that separates the runoff in the southwest portion of the site which gets collected by on-site inlets and piped toward Main Avenue further southwest than the other connection previously mentioned. Therefore, the existing site is divided into three (3) watersheds based upon the direction of surface runoff and available subsurface storm sewer information. However, since the storm conveyance for the majority of the site and the small portion of overland flow to Main Avenue ultimately discharge to the same point, only two (2) design points are studied. Existing Watershed 1 consists of the majority of the site where stormwater runoff is collected and piped to the conveyance system in Main Avenue, Existing Watershed 2 is the north portion of the site that flows to Main Avenue via overland flow, and Existing Watershed 3 is the southwest portion of



the site that is collected and piped to Main Avenue further to the southwest. The area, curve number, and time of concentration for each existing watershed can be seen in the HydroCAD output included in the Appendix.

Under proposed conditions, the site will generally mimic the existing drainage patterns with the two design points. The overall decrease in impervious/gravel cover contributes to the reduction of peak flows, which will be discussed later in the Stormwater Quantity section of this report. New inlets and pipes, as well as some existing inlets and pipes, will be utilized to convey the stormwater runoff adequately to the design points as a result of the proposed improvements. The area, curve number, and time of concentration for each proposed watershed and sub-watershed can be seen in the HydroCAD output included in the Appendix.

#### **STORMWATER QUANTITY:**

1.1 Stormwater Management

As mentioned above, peak flow attenuation of the stormwater runoff at each design point was accomplished due to the decrease in impervious/gravel coverage from predevelopment to post-development conditions. Full HydroCAD output calculations are provided in the appendix. The results are summarized in the table below.

Description	2 Year Storm	10 Year Storm	100 Year Storm
A. Existing Conditions			
Watershed 1 (North/East)	55.30 cfs	86.23 cfs	146.37 cfs
Watershed 2 (Southwest)	11.99 cfs	20.11 cfs	35.86 cfs
B. Proposed Conditions			
Watershed 1 (North/East)	54.33 cfs	85.48 cfs	145.89 cfs
Watershed 2 (Southwest)	11.96 cfs	20.07 cfs	35.78 cfs

#### 1.2 Grand Summary Table

According to 420-4.F(1)(c) in order to control stormwater runoff quantity impacts, the design engineer has three options.

One of the options is to demonstrate through hydrologic and hydraulic analysis that for stormwater leaving the site, post-construction runoff hydrographs for the 2-, 10-, and 100-year storm events do not exceed, at any point in time, the preconstruction runoff hydrographs for the same storm events. Hydrographs comparing the pre- and post-construction discharge for each design point are included in the Appendix of this report demonstrating that this requirement is met.

#### 1.3 Conveyance System

Pipe sizing calculations have been performed using the Rational Method and 25-year design storm for capacity verification. Calculations supporting pipe sizing, including profiles which depict the hydraulic grade line are attached in the Appendix of this report.



#### **STORMWATER QUALITY:**

As per Ordinance §320-4.G.(1), stormwater management measures shall only be required for water quality control if an additional <sup>1</sup>/<sub>4</sub> acre of impervious surface is being proposed on a development site. As this project proposes a reduction of impervious surfaces, stormwater quality is not required for this site.

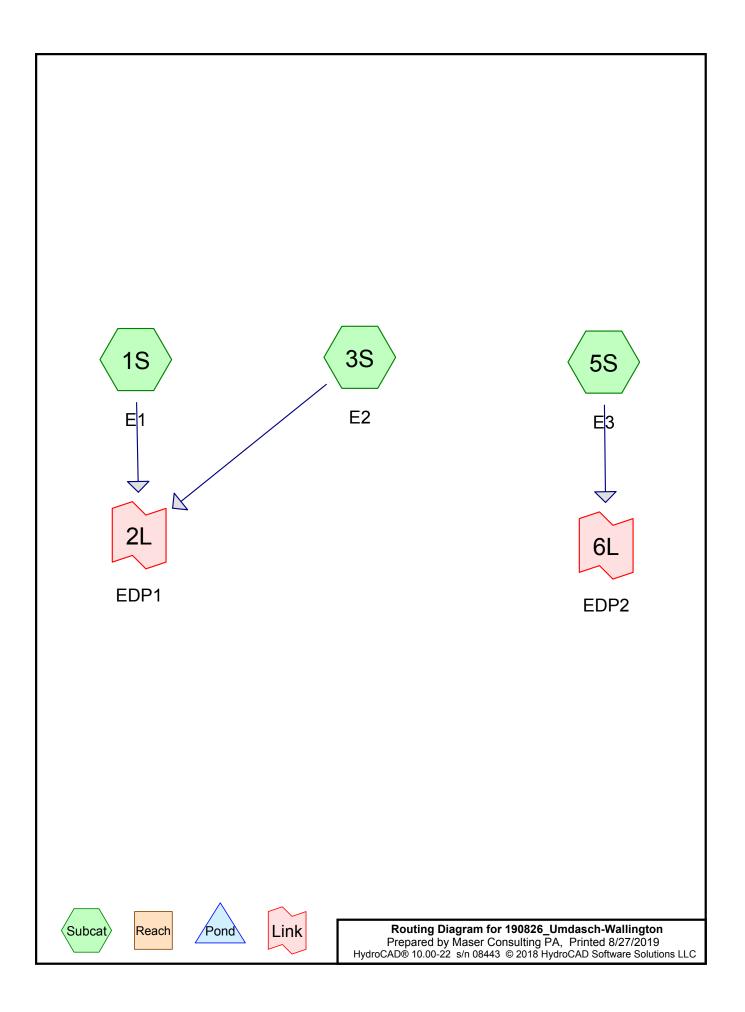
### ANNUAL GROUNDWATER RECHARGE:

As per Ordinance §320-4.F(1)(b)[3], the groundwater recharge requirement does not apply to projects with stormwater from areas of high pollutant loading or industrial stormwater exposed to source material. Since the subject property is an existing contaminated industrial site, the groundwater recharge requirement does not apply.

#### **CONCLUSION & ATTACHMENTS:**

As discussed above, water quantity mitigation is provided for all Watersheds for all storm events studied in accordance with NJDEP regulations. Water quality treatment is not required for this project as the impervious coverage decreases from predevelopment to post-development conditions. Groundwater recharge is not required for this project as the site is contaminated. As such, there should be no adverse impacts due to stormwater, on-site or off-site, as a result of the proposed development. Included in the appendix of this report is the HydroCAD output for back-up on both the existing and proposed conditions. Additionally, the Existing and Proposed Drainage Area Maps, an Inlet Area Map and a Pipe Capacity Report are provided in the appendix as well.

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## Area Listing (selected nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.117	74	>75% Grass cover, Good, HSG C (5S)
0.504	80	>75% Grass cover, Good, HSG D (1S, 3S)
3.344	96	Gravel surface, HSG D (1S)
19.075	98	Paved parking, HSG D (1S, 3S, 5S)
2.374	70	Woods, Good, HSG C (1S, 5S)
0.683	79	Woods/grass comb., Good, HSG D (1S, 3S, 5S)
26.097	94	TOTAL AREA

<b>190826_Umdasch-Wallington</b> Prepared by Maser Consulting PA HydroCAD® 10.00-22 s/n 08443 © 2018 H	Type III 24-hr 2-Year Rainfall=3.34" Printed 8/27/2019 ydroCAD Software Solutions LLC Page 3
Runoff by SCS	0.00-24.00 hrs, dt=0.05 hrs, 481 points TR-20 method, UH=SCS, Weighted-CN +Trans method - Pond routing by Stor-Ind method
Subcatchment1S: E1	Runoff Area=19.794 ac 74.70% Impervious Runoff Depth>2.88" Flow Length=495' Tc=10.4 min CN=96 Runoff=53.29 cfs 4.756 af
Subcatchment3S: E2	Runoff Area=0.891 ac 51.63% Impervious Runoff Depth>2.21" Flow Length=333' Tc=8.2 min CN=89 Runoff=2.10 cfs 0.164 af
Subcatchment5S: E3	Runoff Area=5.412 ac 70.75% Impervious Runoff Depth>2.29" Flow Length=897' Tc=11.4 min CN=90 Runoff=11.99 cfs 1.035 af
Link 2L: EDP1	Inflow=55.30 cfs 4.920 af Primary=55.30 cfs 4.920 af
Link 6L: EDP2	Inflow=11.99 cfs 1.035 af Primary=11.99 cfs 1.035 af
Total Runoff Area = 26.	97 ac Runoff Volume = 5.955 af Average Runoff Depth = 2.74"

26.91% Pervious = 7.022 ac 73.09% Impervious = 19.075 ac

## Summary for Subcatchment 1S: E1

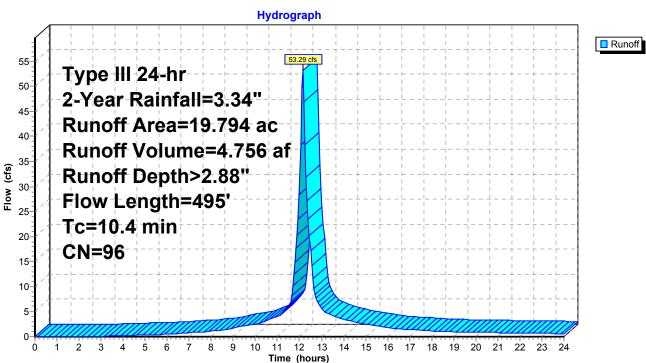
Runoff = 53.29 cfs @ 12.14 hrs, Volume= 4.756 af, Depth> 2.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

_	Area	(ac) C	N Des	cription		
	14.	786 9	8 Pave	ed parking	, HSG D	
	0.	080 8	30 >75	% Grass c	over, Good	, HSG D
	0.	428 7	79 Woo	ods/grass o	comb., Goo	d, HSG D
	1.	156 7	70 Woo	ods, Good,	HSG C	
_	3.	344 9	96 Grav	vel surface	, HSG D	
	19.	794 9	96 Weig	ghted Aver	rage	
	5.	008	25.3	0% Pervio	us Area	
	14.	786	74.7	'0% Imperv	vious Area	
	Тс	Length	Slope	Velocity		Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.1	100	0.2200	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.34"
	0.3	61	0.5578	3.73		Shallow Concentrated Flow,
		. –				Woodland Kv= 5.0 fps
	0.2	17	0.1311	1.81		Shallow Concentrated Flow,
	0.4		0 4004	0.47		Woodland Kv= 5.0 fps
	0.1	11	0.4031	3.17		Shallow Concentrated Flow,
	0.2	05	0.0456	1 22		Woodland Kv= 5.0 fps
	0.3	85	0.0456	4.33		Shallow Concentrated Flow, Paved Kv= 20.3 fps
	1.4	221	0.0166	2.62		Shallow Concentrated Flow,
	1.4	221	0.0100	2.02		Paved Kv= 20.3 fps
-	10.4	405	Total			1 avea 1 (v= 20.0 lp3

10.4 495 Total

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## Subcatchment 1S: E1

#### Summary for Subcatchment 3S: E2

Runoff = 2.10 cfs @ 12.12 hrs, Volume= 0.164 af, Depth> 2.21"

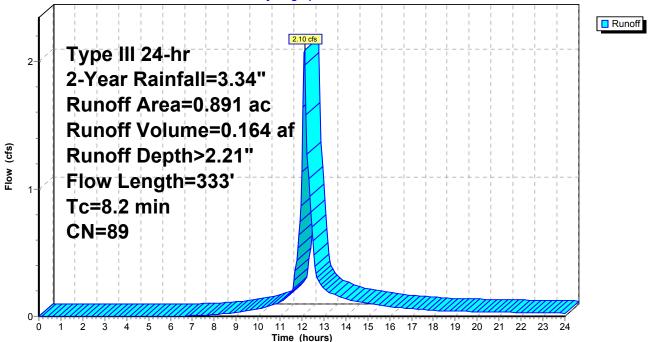
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

_	Area	(ac) C	N Des	cription			
	0.460 98 Paved parking, HSG D						
	0.424 80 >75% Grass cover, Good, HSG D						
	0.	007			comb., Goo	d, HSG D	
_	0.	000	77 Woo	ods, Good,	HSG D		
	0.	891		ghted Aver			
	0.	431	48.3	7% Pervio	us Area		
	0.	460	51.6	3% Imperv	ious Area/		
	Тс	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	7.2	81	0.0270	0.19		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.34"	
	0.3	52	0.0231	3.09		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	
	0.7	200	0.0500	4.54		Shallow Concentrated Flow,	
_						Paved Kv= 20.3 fps	
	~ ~						

8.2 333 Total

## Subcatchment 3S: E2





## Summary for Subcatchment 5S: E3

Runoff = 11.99 cfs @ 12.16 hrs, Volume= 1.035 af, Depth> 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

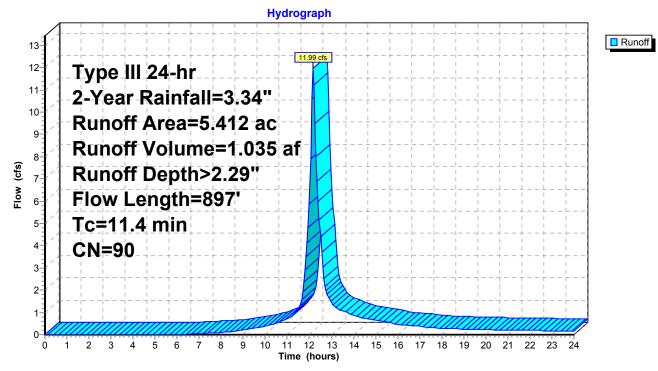
	Area	(ac) C	N Dese	cription					
3.829 98 Paved parking, HSG D									
0.117 74 >75% Grass cover, Good, HSG C 0.248 79 Woods/grass comb., Good, HSG D									
	d, HSG D								
				ds, Good,					
	5.412 90 Weighted Average								
		583	-	5% Pervio					
	3.	829	70.7	5% Imper	ious Area/				
	Тс	Longth	Slone	Volocity	Consoity	Description			
	(min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	5.2	<u>(ieet)</u> 96	0.6110	0.31	(015)	Shoot Flow			
	<b>5.</b> Z	90	0.6110	0.51		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.34"			
	0.1	16	0.4400	3.32		Shallow Concentrated Flow,			
	0.1	10	0.4400	0.02		Woodland Kv= 5.0 fps			
	0.4	71	0.0423	3.31		Shallow Concentrated Flow,			
	0.1	•••	0.0.120	0.01		Unpaved Kv= 16.1 fps			
	0.4	60	0.0167	2.62		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	1.8	171	0.0059	1.56		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	1.2	126	0.0080	1.82		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	0.3	87	0.0576	4.87		Shallow Concentrated Flow,			
		0.45	0.0405			Paved Kv= 20.3 fps			
	1.5	216	0.0139	2.39		Shallow Concentrated Flow,			
	0 5	E 4	0.0005	1 6 4		Paved Kv= 20.3 fps			
	0.5	54	0.0065	1.64		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			

11.4 897 Total

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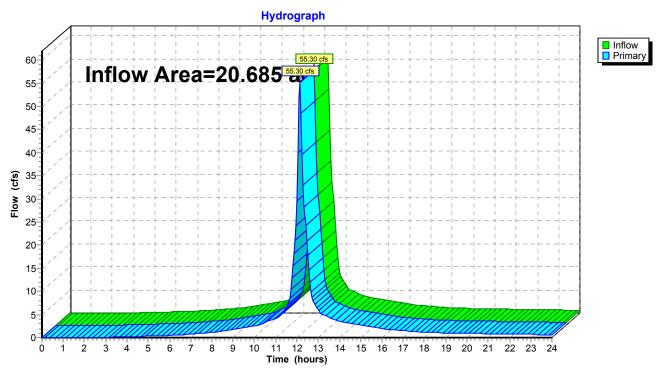




## Summary for Link 2L: EDP1

Inflow Are	a =	20.685 ac, 73.71% Impervious, Inflow Depth > 2.85" for 2-Year e	vent
Inflow	=	55.30 cfs @ 12.14 hrs, Volume= 4.920 af	
Primary	=	55.30 cfs @ 12.14 hrs, Volume= 4.920 af, Atten= 0%, Lag=	0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

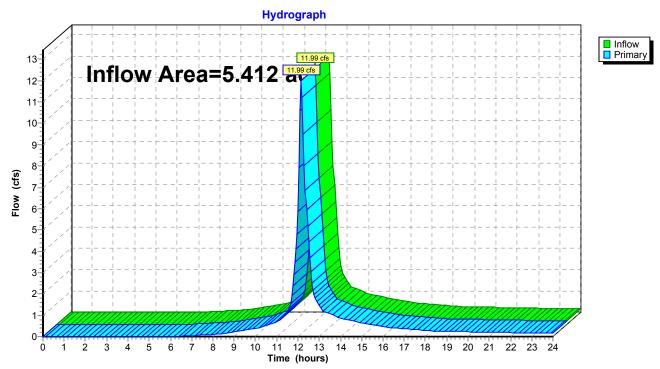


#### Link 2L: EDP1

## Summary for Link 6L: EDP2

Inflow Are	ea =	5.412 ac, 70.75% Impervious, Inflow Depth > 2.29" for 2-Year event
Inflow	=	11.99 cfs @ 12.16 hrs, Volume= 1.035 af
Primary	=	11.99 cfs @ 12.16 hrs, Volume= 1.035 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



#### Link 6L: EDP2

<b>190826_Umdasch-Wallingtor</b> Prepared by Maser Consulting PA HydroCAD® 10.00-22 s/n 08443 © 20	Α	10-Year Rainfall=5.07" Printed 8/27/2019 Page 11
Runoff by	an=0.00-24.00 hrs, dt=0.05 hrs, 481 points SCS TR-20 method, UH=SCS, Weighted-CN or-Ind+Trans method - Pond routing by Stor-Ind	d method
Subcatchment1S: E1	Runoff Area=19.794 ac 74.70% Impervio Flow Length=495' Tc=10.4 min CN=96 R	•
Subcatchment3S: E2	Runoff Area=0.891 ac 51.63% Impervio Flow Length=333' Tc=8.2 min CN=89	
Subcatchment5S: E3	Runoff Area=5.412 ac 70.75% Impervio Flow Length=897' Tc=11.4 min CN=90 R	•
Link 2L: EDP1	-	nflow=86.23 cfs  7.866 af imary=86.23 cfs  7.866 af
Link 6L: EDP2	-	nflow=20.11 cfs  1.776 af imary=20.11 cfs  1.776 af
Total Runoff Area =	26.097 ac Runoff Volume = 9.642 af Avera	age Runoff Depth = 4.43

otal Runoff Area = 26.097 ac Runoff Volume = 9.642 af Average Runoff Depth = 4.43" 26.91% Pervious = 7.022 ac 73.09% Impervious = 19.075 ac

## Summary for Subcatchment 1S: E1

Runoff = 82.83 cfs @ 12.14 hrs, Volume= 7.582 af, Depth> 4.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

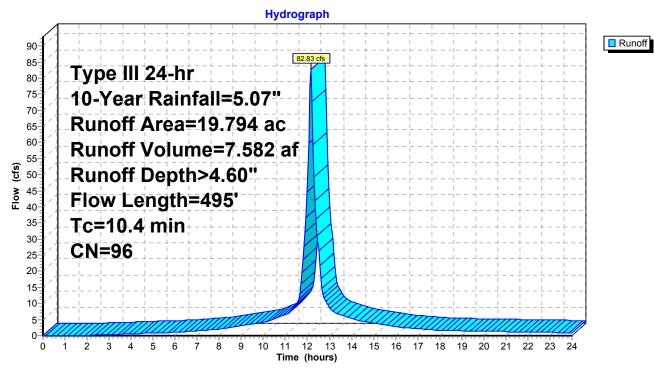
_	Area	(ac) C	N Des	cription		
14.786 98 Paved parking, HSG D						
	0.	080 E	30 >75	% Grass c	over, Good	, HSG D
	0.	428 7	79 Woo	ods/grass o	comb., Goo	d, HSG D
	1.	156 7	70 Woo	ods, Good,	HSG C	
_	3.	344 9	96 Grav	vel surface	, HSG D	
	19.	794 9	96 Weig	ghted Aver	age	
	5.	800	25.3	0% Pervio	us Area	
	14.	786	74.7	'0% Imper	ious Area/	
	_				_	
	Тс	Length	Slope	Velocity		Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.1	100	0.2200	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.34"
	0.3	61	0.5578	3.73		Shallow Concentrated Flow,
		47	0 4044	4.04		Woodland Kv= 5.0 fps
	0.2	17	0.1311	1.81		Shallow Concentrated Flow,
	0.4	4.4	0 4004	0 47		Woodland Kv= 5.0 fps
	0.1	11	0.4031	3.17		Shallow Concentrated Flow,
	0.3	85	0.0456	4.33		Woodland Kv= 5.0 fps Shallow Concentrated Flow,
	0.5	60	0.0400	4.55		Paved Kv= 20.3 fps
	1.4	221	0.0166	2.62		Shallow Concentrated Flow,
	1.4		0.0100	2.02		Paved Kv= 20.3 fps
-	10.4	105	Total			

10.4 495 Total

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Prepared by Maser Consulting PA HydroCAD® 10.00-22 s/n 08443 © 2018 HydroCAD Software Solutions LLC





#### Summary for Subcatchment 3S: E2

Runoff = 3.57 cfs @ 12.11 hrs, Volume= 0.285 af, Depth> 3.83"

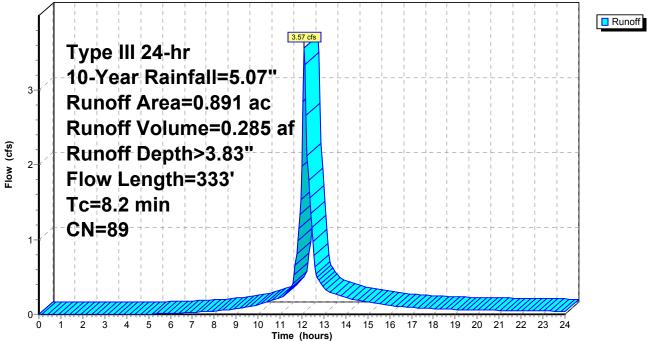
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

_	Area	(ac) C	N Des	cription		
	0.460 98 Paved parking, HSG D					
	0.	424	80 >75	% Grass c	over, Good	, HSG D
	0.	007	79 Woo	ods/grass o	comb., Goo	d, HSG D
_	0.	000	77 Woo	ods, Good,	HSG D	
	0.	891	89 Weig	ghted Aver	rage	
	0.	431	48.3	7% Pervio	us Area	
	0.	460	51.6	3% Imperv	vious Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.2	81	0.0270	0.19		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.34"
	0.3	52	0.0231	3.09		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.7	200	0.0500	4.54		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps
	~ ~					

8.2 333 Total

## Subcatchment 3S: E2





## Summary for Subcatchment 5S: E3

Runoff = 20.11 cfs @ 12.16 hrs, Volume= 1.776 af, Depth> 3.94"

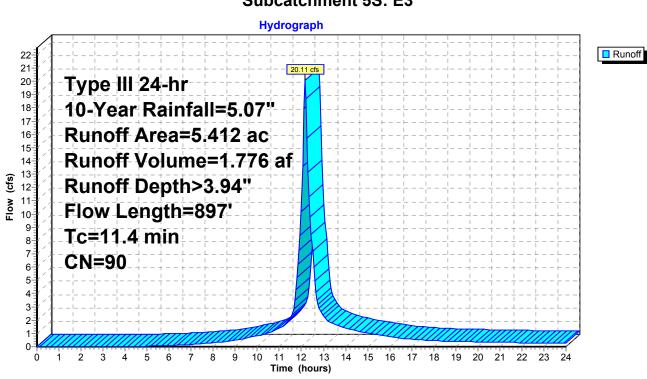
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

	Area	(ac) C	N Des	cription		
	3.	829 9	8 Pave	ed parking	, HSG D	
	0.117 74 >75% Grass cover, Good,					
				•	comb., Goo	d, HSG D
_				ds, Good,	HSG C	
	-	-		ghted Aver		
		583		5% Pervio		
	3.	829	70.7	5% Imper	ious Area/	
	Та	Longth	Clana	Volocity	Conosity	Description
	Tc (min)	Length	Slope (ft/ft)	Velocity	Capacity	Description
_	(min)	(feet)	. ,	(ft/sec)	(cfs)	
	5.2	96	0.6110	0.31		Sheet Flow,
	0.1	16	0.4400	3.32		Woods: Light underbrush n= 0.400 P2= 3.34" Shallow Concentrated Flow,
	0.1	10	0.4400	5.52		Woodland Kv= 5.0 fps
	0.4	71	0.0423	3.31		Shallow Concentrated Flow,
	0.1		0.0120	0.01		Unpaved Kv= 16.1 fps
	0.4	60	0.0167	2.62		Shallow Concentrated Flow,
	-			-		Paved Kv= 20.3 fps
	1.8	171	0.0059	1.56		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	1.2	126	0.0080	1.82		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.3	87	0.0576	4.87		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	1.5	216	0.0139	2.39		Shallow Concentrated Flow,
	0.5	<b>F</b> 4	0.0005	4.04		Paved Kv= 20.3 fps
	0.5	54	0.0065	1.64		Shallow Concentrated Flow,
_		907	Total			Paved Kv= 20.3 fps

11.4 897 Total

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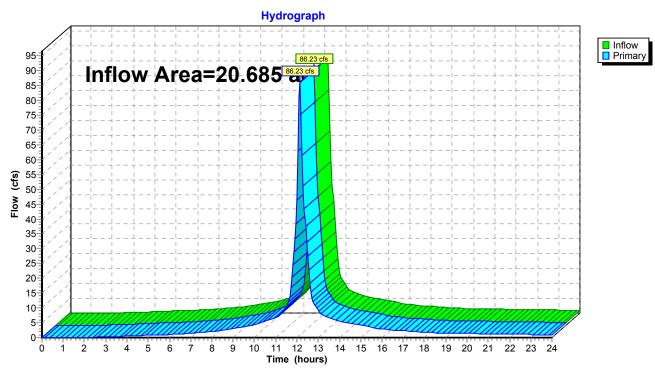


## Subcatchment 5S: E3

## Summary for Link 2L: EDP1

Inflow Area	a =	20.685 ac, 73.71% Impervious, Inflow Depth > 4.56" for 10-Year event
Inflow	=	86.23 cfs @ 12.14 hrs, Volume= 7.866 af
Primary	=	86.23 cfs @ 12.14 hrs, Volume= 7.866 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

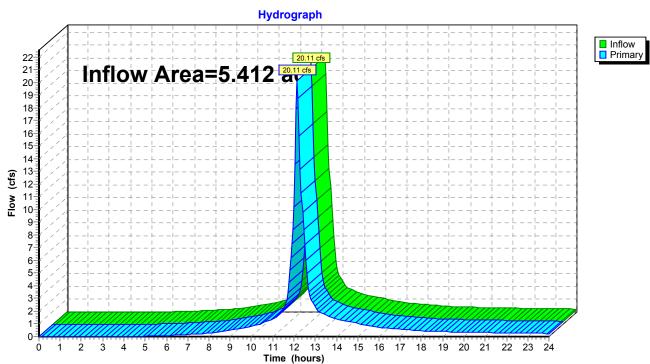


#### Link 2L: EDP1

## Summary for Link 6L: EDP2

Inflow Are	ea =	5.412 ac, 70.75% Impervious, Inflow Depth > 3.94" for 10-Year event
Inflow	=	20.11 cfs @ 12.16 hrs, Volume= 1.776 af
Primary	=	20.11 cfs @ 12.16 hrs, Volume= 1.776 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



### Link 6L: EDP2

<b>190826_Umdasch-Wallington</b> Prepared by Maser Consulting PA	<i>Type III 24-hr 100-Year Rainfall=8.47"</i> Printed 8/27/2019
HydroCAD® 10.00-22 s/n 08443 © 2018	HydroCAD Software Solutions LLC Page 19
Runoff by SC	0.00-24.00 hrs, dt=0.05 hrs, 481 points S TR-20 method, UH=SCS, Weighted-CN d+Trans method - Pond routing by Stor-Ind method
Subcatchment1S: E1	Runoff Area=19.794 ac 74.70% Impervious Runoff Depth>7.98" Flow Length=495' Tc=10.4 min CN=96 Runoff=140.25 cfs 13.163 af
Subcatchment3S: E2	Runoff Area=0.891 ac 51.63% Impervious Runoff Depth>7.14" Flow Length=333' Tc=8.2 min CN=89 Runoff=6.42 cfs 0.530 af
Subcatchment5S: E3	Runoff Area=5.412 ac 70.75% Impervious Runoff Depth>7.26" Flow Length=897' Tc=11.4 min CN=90 Runoff=35.86 cfs 3.273 af
Link 2L: EDP1	Inflow=146.37 cfs 13.693 af Primary=146.37 cfs 13.693 af
Link 6L: EDP2	Inflow=35.86 cfs 3.273 af Primary=35.86 cfs 3.273 af
Total Runoff Area = 26.0	97 ac Runoff Volume = 16.966 af Average Runoff Depth = 7.80" 26.91% Pervious = 7.022 ac 73.09% Impervious = 19.075 ac

## Summary for Subcatchment 1S: E1

Runoff = 140.25 cfs @ 12.14 hrs, Volume= 13.163 af, Depth> 7.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

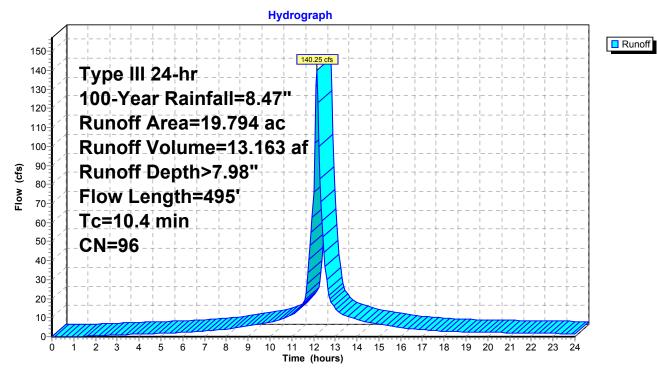
	Area	(ac) C	N Des	cription		
14.786 98			8 Pave	ed parking	, HSG D	
0.080 80			30 >75	% Grass c	over, Good	, HSG D
	0.4	428 7	79 Woo	ds/grass d	comb., Goo	d, HSG D
	1.	156 7	70 Woo	ds, Good,	HSG C	
_	3.	344 9	96 Grav	el surface	, HSG D	
	19.	794 9	96 Weig	ghted Aver	age	
	5.	800	25.3	0% Pervio	us Area	
	14.	786	74.7	0% Imperv	ious Area/	
	Тс	Length	Slope	Velocity	• •	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	8.1	100	0.2200	0.21		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.34"
	0.3	61	0.5578	3.73		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.2	17	0.1311	1.81		Shallow Concentrated Flow,
	0.4		0 4004	0.47		Woodland Kv= 5.0 fps
	0.1	11	0.4031	3.17		Shallow Concentrated Flow,
	0.0	05	0.0450	4 0 0		Woodland Kv= 5.0 fps
	0.3	85	0.0456	4.33		Shallow Concentrated Flow,
	1.4	221	0.0166	2.62		Paved Kv= 20.3 fps Shallow Concentrated Flow,
	1.4	221	0.0100	2.02		Paved Kv= 20.3 fps
	10.4	405	Total			

10.4 495 Total

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Subcatchment 1S: E1



#### Summary for Subcatchment 3S: E2

Runoff = 6.42 cfs @ 12.11 hrs, Volume= 0.530 af, Depth> 7.14"

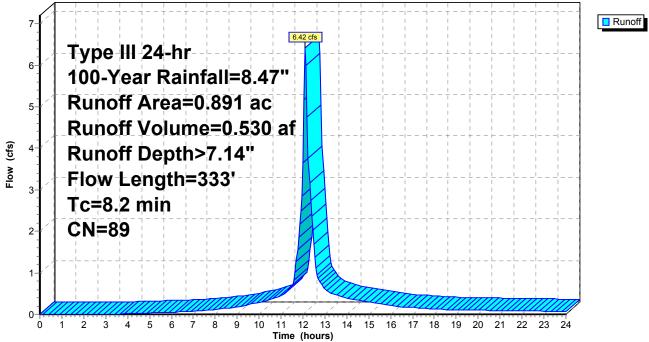
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

_	Area	(ac) C	N Des	cription		
	0.460 98 Paved parking, HSG D					
	0.	424 8	30 >75°	% Grass c	over, Good	, HSG D
	0.	007	79 Woo	ods/grass o	comb., Goo	d, HSG D
_	0.	000	77 Woo	ods, Good,	HSG D	
	0.	891 8		ghted Aver		
	-	431		7% Pervio		
	0.	460	51.6	3% Imperv	ious Area/	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	7.2	81	0.0270	0.19		Sheet Flow,
	0.3	52	0.0231	3.09		Grass: Short n= 0.150 P2= 3.34" Shallow Concentrated Flow, Paved Kv= 20.3 fps
	0.7	200	0.0500	4.54		Shallow Concentrated Flow,
_						Paved Kv= 20.3 fps

8.2 333 Total

## Subcatchment 3S: E2





## Summary for Subcatchment 5S: E3

Runoff = 35.86 cfs @ 12.15 hrs, Volume= 3.273 af, Depth> 7.26"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

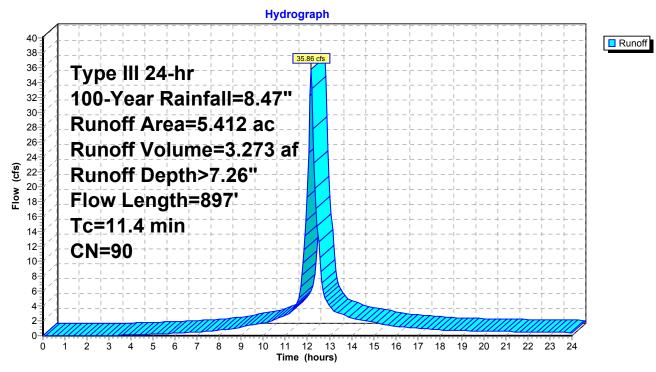
3.829 98 Paved parking, HSG D	
0.117 74 >75% Grass cover, Good, HSG C	
0.248 79 Woods/grass comb., Good, HSG D	
1.218 70 Woods, Good, HSG C	
5.412 90 Weighted Average	
1.583 29.25% Pervious Area	
3.829 70.75% Impervious Area	
To Longth Slong Velocity Conscity Description	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5.2 96 0.6110 0.31 <b>Sheet Flow,</b>	- 2 24"
0.1 16 0.4400 3.32 Woods: Light underbrush n= 0.400 P2= <b>Shallow Concentrated Flow,</b>	- 3.34
Woodland Kv= 5.0 fps	
0.4 71 0.0423 3.31 Shallow Concentrated Flow,	
Unpaved Kv= 16.1 fps	
0.4 60 0.0167 2.62 Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	
1.8 171 0.0059 1.56 Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	
1.2 126 0.0080 1.82 Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	
0.3 87 0.0576 4.87 Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	
1.5         216         0.0139         2.39         Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	
0.5 54 0.0065 1.64 Shallow Concentrated Flow,	
Paved Kv= 20.3 fps	

11.4 897 Total

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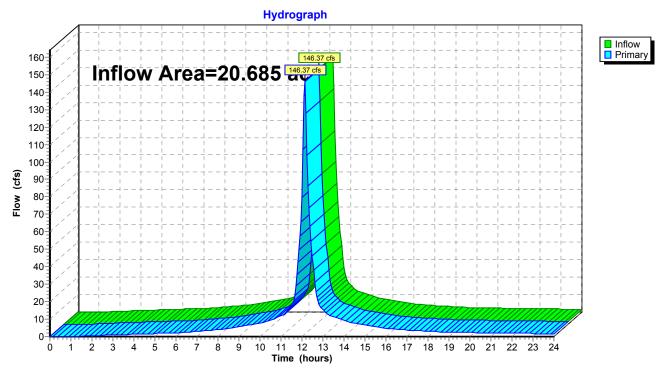




## Summary for Link 2L: EDP1

Inflow Are	ea =	20.685 ac, 73.71% Impervious, Inflow Depth > 7.94" for 100-Year event
Inflow	=	146.37 cfs @ 12.14 hrs, Volume= 13.693 af
Primary	=	146.37 cfs @ 12.14 hrs, Volume= 13.693 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

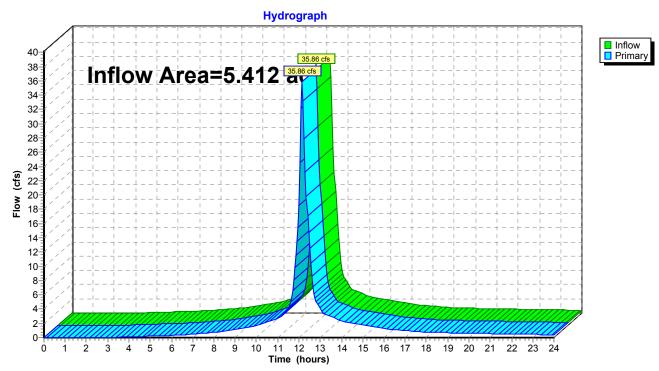


#### Link 2L: EDP1

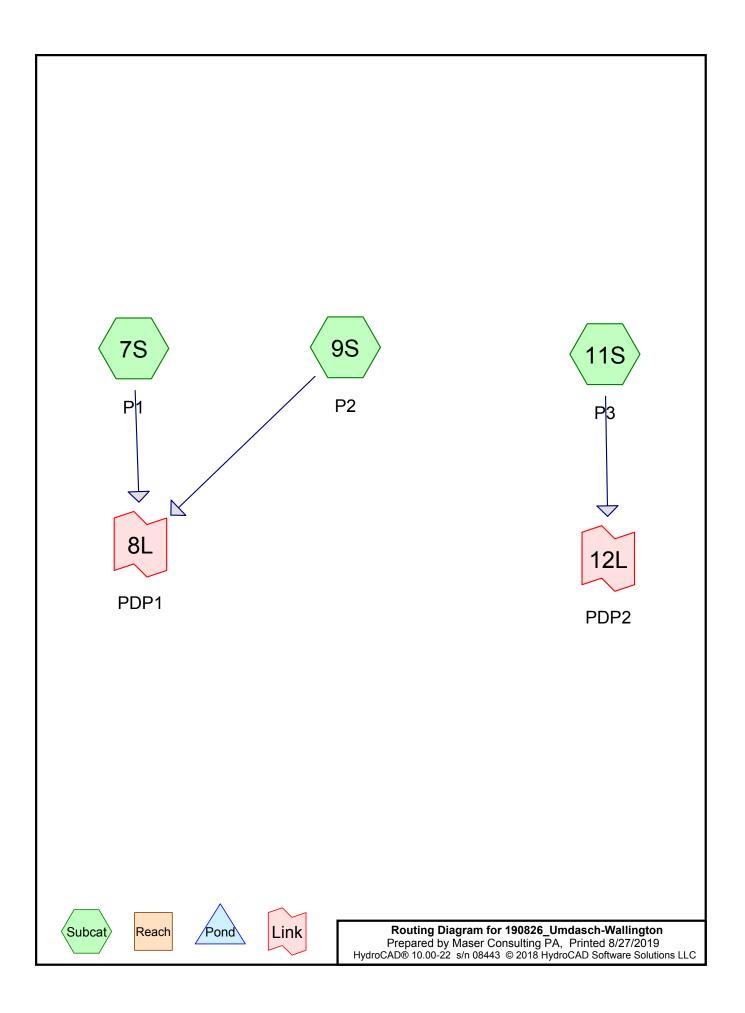
## Summary for Link 6L: EDP2

Inflow Are	a =	5.412 ac, 70.75% Impervious, Inflow Depth > 7.26" for 100-Year event	
Inflow	=	35.86 cfs @ 12.15 hrs, Volume= 3.273 af	
Primary	=	35.86 cfs @ 12.15 hrs, Volume= 3.273 af, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



#### Link 6L: EDP2



## Area Listing (selected nodes)

Area	CN	Description	
(acres)		(subcatchment-numbers)	
0.325	74	>75% Grass cover, Good, HSG C (7S, 11S)	
2.264	80	>75% Grass cover, Good, HSG D (7S, 9S, 11S)	
21.419	98	Paved parking, HSG D (7S, 9S, 11S)	
1.685	70	Woods, Good, HSG C (7S, 11S)	
0.404	79	Woods/grass comb., Good, HSG D (7S, 11S)	
26.097	94	TOTAL AREA	

<b>190826_Umdasch-Wallington</b> Prepared by Maser Consulting PA	<i>Type III 24-hr 2-Year Rainfall=3.34"</i> Printed 8/27/2019
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Time spar Runoff by S	n=0.00-24.00 hrs, dt=0.05 hrs, 481 points CS TR-20 method, UH=SCS, Weighted-CN Ind+Trans method - Pond routing by Stor-Ind method
Subcatchment7S: P1 Flow Length=	Runoff Area=20.449 ac 86.26% Impervious Runoff Depth>2.78" 105' Slope=0.0165 '/' Tc=10.4 min CN=95 Runoff=53.86 cfs 4.734 af
Subcatchment9S: P2	Runoff Area=0.262 ac 15.27% Impervious Runoff Depth>1.72" Flow Length=127' Tc=8.2 min CN=83 Runoff=0.48 cfs 0.038 af
Subcatchment11S: P3	Runoff Area=5.386 ac 69.44% Impervious Runoff Depth>2.29" Flow Length=631' Tc=11.3 min CN=90 Runoff=11.96 cfs 1.030 af
Link 8L: PDP1	Inflow=54.33 cfs 4.772 af Primary=54.33 cfs 4.772 af
Link 12L: PDP2	Inflow=11.96 cfs 1.030 af Primary=11.96 cfs 1.030 af
Total Runoff Area = 2	26.097 ac Runoff Volume = 5.802 af Average Runoff Depth = 2.67

tal Runoff Area = 26.097 ac Runoff Volume = 5.802 af Average Runoff Depth = 2.67" 17.93% Pervious = 4.678 ac 82.07% Impervious = 21.419 ac

#### Summary for Subcatchment 7S: P1

Runoff = 53.86 cfs @ 12.14 hrs, Volume= 4.734 af, Depth> 2.78"

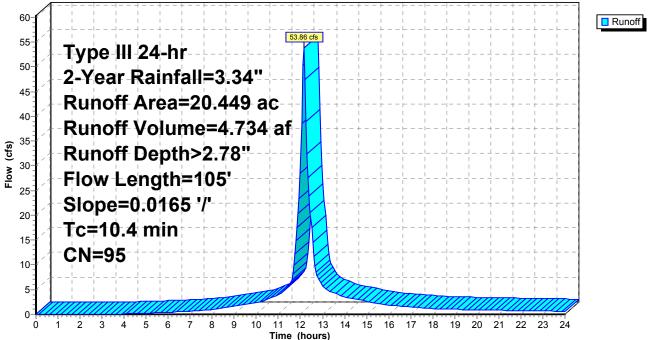
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

_	Area (ac)		CN	Description				
	17.	639	98	Pave	Paved parking, HSG D			
0.000 96				Grav	Gravel surface, HSG D			
0.219 7				>75%	>75% Grass cover, Good, HSG C			
	1.786			>75%	>75% Grass cover, Good, HSG D			
	0.266			Woo	Woods/grass comb., Good, HSG D			
	0.	539	70	Woods, Good, HSG C				
20.449 95 Weighted Average								
2.810 13.74% Pervious Area								
	17.	639		86.26% Impervious Area				
	Тс	Lengt	h	Slope	Velocity	Capacity	Description	
_	(min) (feet		t)	(ft/ft)	(ft/sec)	(cfs)		
	10.4 1		0 0	0.0165	0.16		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.34"	
	0.0		5 0	0.0165	2.07		Shallow Concentrated Flow,	
							Unpaved Kv= 16.1 fps	

10.4 105 Total

## Subcatchment 7S: P1





#### Summary for Subcatchment 9S: P2

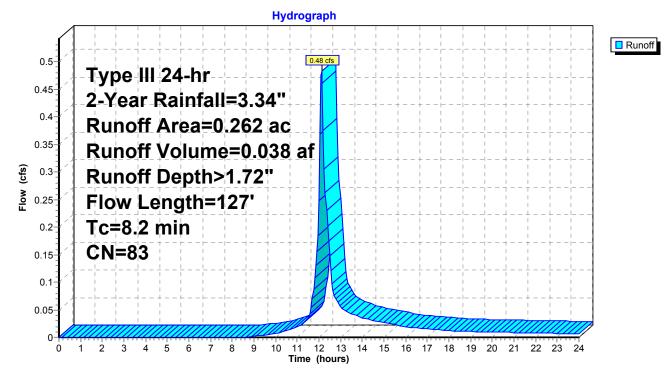
Runoff = 0.48 cfs @ 12.12 hrs, Volume= 0.038 af, Depth> 1.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

_	Area (ac) CN Description					
	0.	040 9	98 Pave	ed parking	, HSG D	
_	0.	222 8	30 >75°	% Grass c	over, Good	, HSG D
	0.	262 8	33 Weig	ghted Aver	age	
	0.	222	84.7	3% Pervio	us Area	
	0.	040	15.2	7% Imperv	vious Area	
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	p
_	8.1	88	0.0235	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.34"
	0.1	39	0.2820	8.55		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	82	127	Total			

8.2 127 Total

### Subcatchment 9S: P2



### Summary for Subcatchment 11S: P3

Runoff = 11.96 cfs @ 12.16 hrs, Volume= 1.030 af, Depth> 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.34"

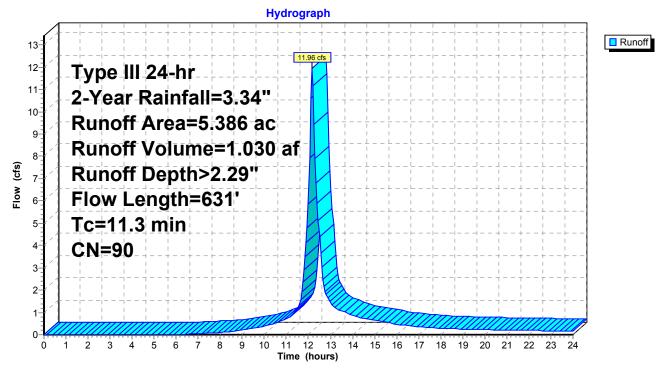
Area	(ac) C	N Dese	cription							
3.	740 9	8 Pave	ed parking	, HSG D						
0.	000 9	96 Grav	el surface	, HSG D						
0.	106 7	74 >75 <sup>°</sup>	% Grass co	over, Good	, HSG C					
-			, HSG D							
0.	0.138 79 Woods/grass comb., Good, HSG D									
1.	146 7	70 Woo	ds, Good,	HSG C						
5.	386 9	90 Weig	ghted Aver	age						
1.	646	30.5	6% Pervio	us Area						
3.	740	69.4	4% Imperv	∕ious Area						
_										
Tc	Length	Slope		Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
6.2	100	0.4200	0.27		Sheet Flow,					
					Woods: Light underbrush n= 0.400 P2= 3.34"					
0.1	19	0.4700	3.43		Shallow Concentrated Flow,					
0.4	07	0.0070	0.40		Woodland Kv= 5.0 fps					
0.1	27	0.0376	3.12		Shallow Concentrated Flow,					
0.5	65	0.0452	1 00		Unpaved Kv= 16.1 fps					
0.5	65	0.0153	1.99		Shallow Concentrated Flow,					
1.0	100	0.0100	1.61		Unpaved Kv= 16.1 fps Shallow Concentrated Flow,					
1.0	100	0.0100	1.01		Unpaved Kv= 16.1 fps					
1.8	143	0.0070	1.35		Shallow Concentrated Flow,					
1.0	140	0.0070	1.00		Unpaved Kv= 16.1 fps					
0.6	70	0.0143	1.93		Shallow Concentrated Flow,					
0.0	,0	0.0140	1.00		Unpaved Kv= 16.1 fps					
1.0	107	0.0079	1.80		Shallow Concentrated Flow,					
		5.0010			Paved Kv= 20.3 fps					
11 3	631	Total			· · · · · · · · · · · · · · · · · · ·					

11.3 631 Total

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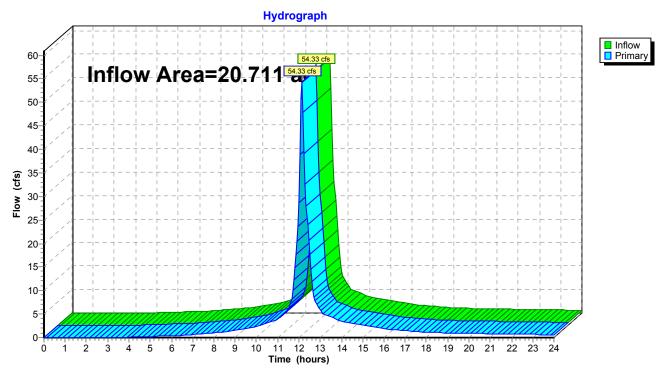




## Summary for Link 8L: PDP1

Inflow Are	a =	20.711 ac, 85.36% Impervious, Inflow Depth > 2.76" for 2-Year event	
Inflow	=	54.33 cfs @ 12.14 hrs, Volume= 4.772 af	
Primary	=	54.33 cfs @ 12.14 hrs, Volume= 4.772 af, Atten= 0%, Lag= 0.0 m	nin

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

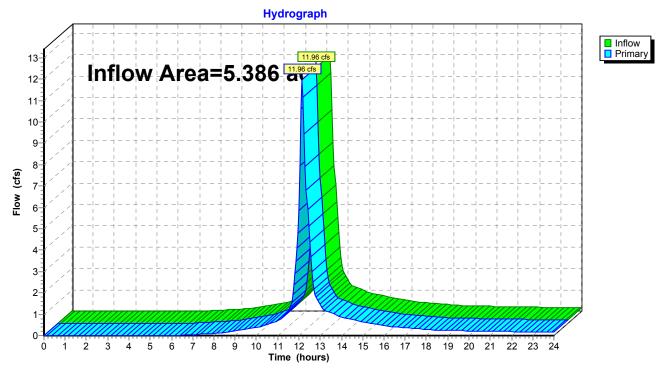


#### Link 8L: PDP1

## Summary for Link 12L: PDP2

Inflow Are	a =	5.386 ac, 69.44% Impervious, Inflow Depth > 2.29" for 2-Year event
Inflow	=	11.96 cfs @ 12.16 hrs, Volume= 1.030 af
Primary	=	11.96 cfs @ 12.16 hrs, Volume= 1.030 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



### Link 12L: PDP2

<b>190826_Umdasch-Wallington</b> Prepared by Maser Consulting PA	<i>Type III 24-hr 10-Year Rainfall=5.07"</i> Printed 8/27/2019
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Runoff by SC	0.00-24.00 hrs, dt=0.05 hrs, 481 points S TR-20 method, UH=SCS, Weighted-CN d+Trans method - Pond routing by Stor-Ind method
Subcatchment7S: P1 Flow Length=10	Runoff Area=20.449 ac 86.26% Impervious Runoff Depth>4.48" 5' Slope=0.0165 '/' Tc=10.4 min CN=95 Runoff=84.61 cfs 7.639 af
Subcatchment9S: P2	Runoff Area=0.262 ac 15.27% Impervious Runoff Depth>3.23" Flow Length=127' Tc=8.2 min CN=83 Runoff=0.91 cfs 0.071 af
Subcatchment11S: P3	Runoff Area=5.386 ac 69.44% Impervious Runoff Depth>3.94" Flow Length=631' Tc=11.3 min CN=90 Runoff=20.07 cfs 1.767 af
Link 8L: PDP1	Inflow=85.48 cfs 7.710 af
	Primary=85.48 cfs 7.710 af
Link 12L: PDP2	Inflow=20.07 cfs 1.767 af
	Primary=20.07 cfs 1.767 af
Total Runoff Area = 26	097 ac Runoff Volume = 9 477 af Average Runoff Depth = 4 36

Total Runoff Area = 26.097 ac Runoff Volume = 9.477 af Average Runoff Depth = 4.36" 17.93% Pervious = 4.678 ac 82.07% Impervious = 21.419 ac

#### Summary for Subcatchment 7S: P1

Runoff = 84.61 cfs @ 12.14 hrs, Volume= 7.639 af, Depth> 4.48"

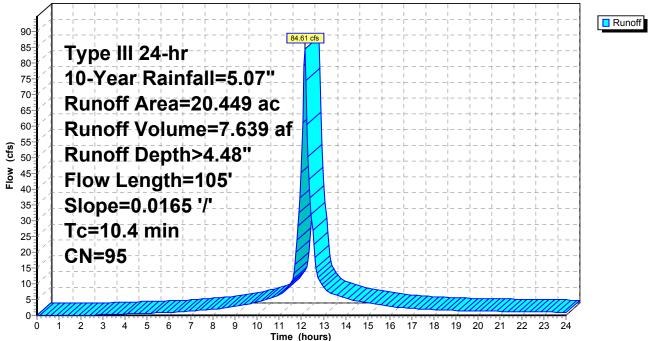
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

_	Area (ac) CN Description							
	17.639 98 Paved parking, HSG D							
	0.	000	96	Grav	el surface	, HSG D		
	0.	219	74	>75%	% Grass co	over, Good	, HSG C	
	1.	786	80	>75%	% Grass co	over, Good	, HSG D	
	0.	266	79	Woo	ds/grass c	omb., Goo	d, HSG D	
_	0.	539	70	Woo	ds, Good,	HSG C		
	20.	449	95	Weig	phted Aver	age		
	2.	810		13.7	4% Pervio	us Area		
	17.	639		86.2	6% Imperv	vious Area		
	Тс	Lengt	า :	Slope	Velocity	Capacity	Description	
_	(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)		
	10.4	10	0 0	.0165	0.16		Sheet Flow,	
							Grass: Short n= 0.150 P2= 3.34"	
	0.0	ļ	50	.0165	2.07		Shallow Concentrated Flow,	
							Unpaved Kv= 16.1 fps	

10.4 105 Total

## Subcatchment 7S: P1





#### Summary for Subcatchment 9S: P2

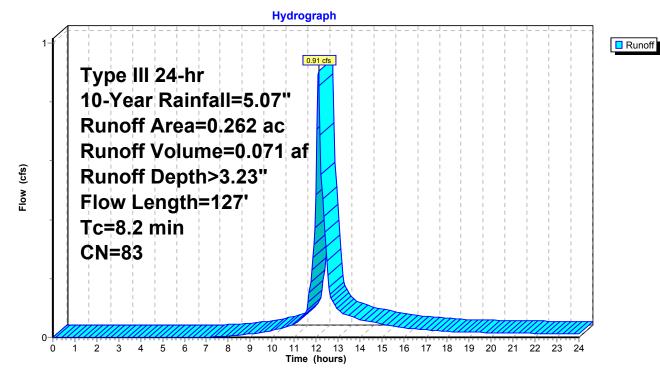
Runoff = 0.91 cfs @ 12.12 hrs, Volume= 0.071 af, Depth> 3.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

_	Area	Area (ac) CN Description					
	0.	040	98 Pav	ed parking	, HSG D		
_	0.	222	80 >75	% Grass c	over, Good	, HSG D	
	0.	262	83 Wei	ghted Avei	age		
	0.	222	84.7	3% Pervio	us Area		
	0.	040	15.2	7% Imper	vious Area		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	8.1	88	0.0235	0.18		Sheet Flow,	
_	0.1	39	0.2820	8.55		Grass: Short n= 0.150 P2= 3.34" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	8.2	127	Total				

8.2 127 Total

#### Subcatchment 9S: P2



### Summary for Subcatchment 11S: P3

Runoff = 20.07 cfs @ 12.15 hrs, Volume= 1.767 af, Depth> 3.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.07"

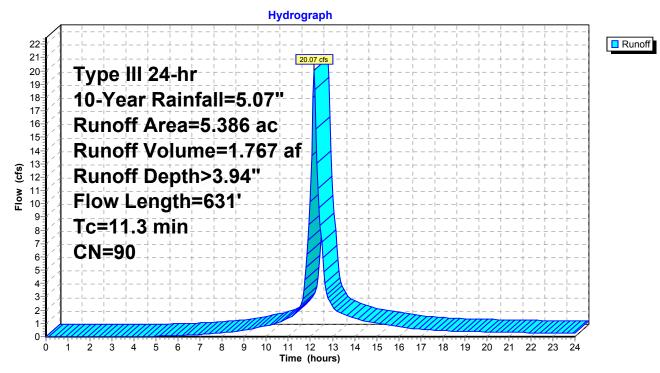
_	Area	(ac) C	N Desc	cription					
	3.	740 9	8 Pave	ed parking	, HSG D				
	0.	000 9	6 Grav	el surface	, HSG D				
					over, Good				
	-				over, Good				
0.138 79 Woods/grass comb., Good, HSG D									
_	1.	<u>146 7</u>	'0 Woo	ds, Good,	HSG C				
			0 Weig	ghted Aver	age				
		646		6% Pervio					
	3.	740	69.4	4% Imper	ious Area				
	-		01		<b>o</b>				
	Tc	Length	Slope		Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	6.2	100	0.4200	0.27		Sheet Flow,			
	0.4	10	0 4700	0.40		Woods: Light underbrush n= 0.400 P2= 3.34"			
	0.1	19	0.4700	3.43		Shallow Concentrated Flow,			
	0.4	07	0 0070	0.40		Woodland Kv= 5.0 fps			
	0.1	27	0.0376	3.12		Shallow Concentrated Flow,			
	0.5	6F	0.0153	1.99		Unpaved Kv= 16.1 fps			
	0.5	65	0.0155	1.99		Shallow Concentrated Flow,			
	1.0	100	0.0100	1.61		Unpaved Kv= 16.1 fps Shallow Concentrated Flow,			
	1.0	100	0.0100	1.01		Unpaved Kv= 16.1 fps			
	1.8	143	0.0070	1.35		Shallow Concentrated Flow,			
	1.0	175	0.0070	1.00		Unpaved Kv= 16.1 fps			
	0.6	70	0.0143	1.93		Shallow Concentrated Flow,			
	0.0	.0	0.0110	1.00		Unpaved Kv= 16.1 fps			
	1.0	107	0.0079	1.80		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
-	11 3	631	Total						

11.3 631 Total

#### 190826\_Umdasch-Wallington

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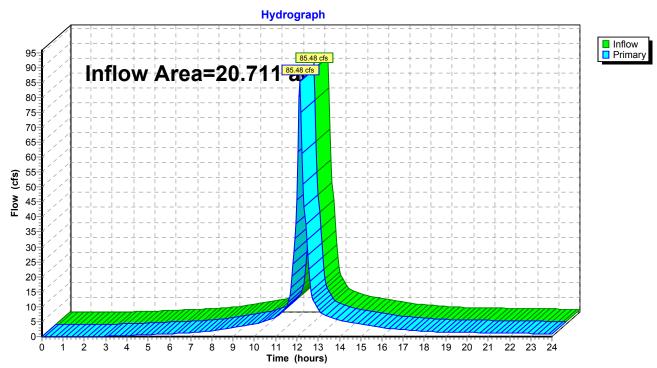
Subcatchment 11S: P3



## Summary for Link 8L: PDP1

Inflow Are	a =	20.711 ac, 85.36% Impervious, Inflow Depth > 4.47" for 10-Year event	
Inflow	=	85.48 cfs @ 12.14 hrs, Volume= 7.710 af	
Primary	=	85.48 cfs @ 12.14 hrs, Volume= 7.710 af, Atten= 0%, Lag= 0.0 min	۱

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

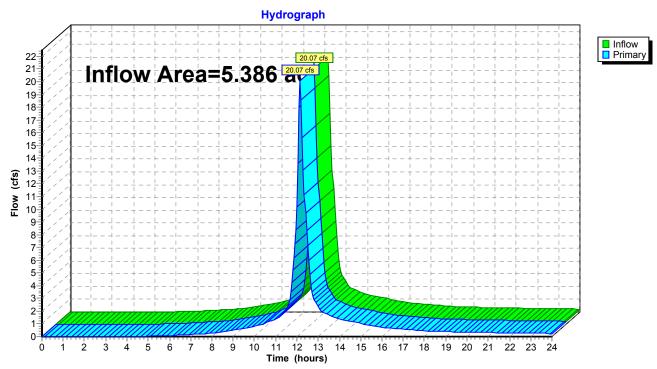


#### Link 8L: PDP1

## Summary for Link 12L: PDP2

Inflow Are	a =	5.386 ac, 69.44% Impervious, Inflow Depth > 3.94" for 10-Year event
Inflow	=	20.07 cfs @ 12.15 hrs, Volume= 1.767 af
Primary	=	20.07 cfs @ 12.15 hrs, Volume= 1.767 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs



### Link 12L: PDP2

<b>190826_Umdasch-Wallington</b> Prepared by Maser Consulting PA HydroCAD® 10.00-22 s/n 08443 © 2018	Type III 24-hr 100-Year Rainfall=8.47" Printed 8/27/2019						
TIYUIOCAD® 10:00-22 \$/11 08443 @ 2018	HydroCAD Software Solutions LLC Page 17						
Time span=0.00-24.00 hrs, dt=0.05 hrs, 481 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method							
Subcatchment7S: P1 Flow Length=105	Runoff Area=20.449 ac 86.26% Impervious Runoff Depth>7.86" Slope=0.0165 '/' Tc=10.4 min CN=95 Runoff=144.21 cfs 13.394 af						
Subcatchment9S: P2	Runoff Area=0.262 ac 15.27% Impervious Runoff Depth>6.42" Flow Length=127' Tc=8.2 min CN=83 Runoff=1.75 cfs 0.140 af						
Subcatchment11S: P3	Runoff Area=5.386 ac 69.44% Impervious Runoff Depth>7.26" Flow Length=631' Tc=11.3 min CN=90 Runoff=35.78 cfs 3.258 af						
Link 8L: PDP1	Inflow=145.89 cfs 13.534 af						
	Primary=145.89 cfs 13.534 af						
Link 12L: PDP2	Inflow=35.78 cfs 3.258 af						
	Primary=35.78 cfs 3.258 af						
Total Runoff Area = 26.	097 ac Runoff Volume = 16.791 af Average Runoff Depth = 7.72						

Total Runoff Area = 26.097 ac Runoff Volume = 16.791 af Average Runoff Depth = 7.72" 17.93% Pervious = 4.678 ac 82.07% Impervious = 21.419 ac

#### Summary for Subcatchment 7S: P1

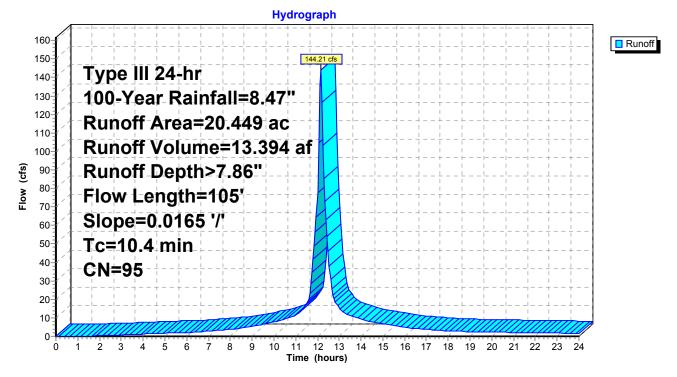
Runoff = 144.21 cfs @ 12.14 hrs, Volume= 13.394 af, Depth> 7.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

Area	Area (ac) CN Description						
17.	17.639 98 Paved parking, HSG D						
0.	000	96	Grav	el surface	, HSG D		
0.	219	74	>75%	% Grass co	over, Good	, HSG C	
1.	786	80	>75%	% Grass co	over, Good	, HSG D	
0.	266	79	Woo	ds/grass c	comb., Goo	d, HSG D	
0.	539	70	Woo	ds, Good,	HSG C		
20.	449	95	Weig	hted Aver	age		
2.	810		13.7	4% Pervio	us Area		
17.	639		86.2	6% Imperv	ious Area		
Тс	Length	n S	Slope	Velocity	Capacity	Description	
(min)	(feet	)	(ft/ft)	(ft/sec)	(cfs)		
10.4	100	0.0	0165	0.16		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.34"	
0.0	5	5 0.0	0165	2.07		Shallow Concentrated Flow,	
						Unpaved Kv= 16.1 fps	

10.4 105 Total

### Subcatchment 7S: P1



#### Summary for Subcatchment 9S: P2

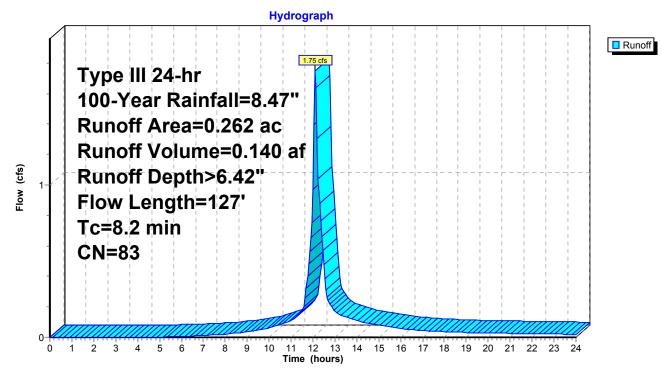
Runoff = 1.75 cfs @ 12.11 hrs, Volume= 0.140 af, Depth> 6.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

_	Area	(ac) C	N Des	cription		
	0.	040	98 Pave	ed parking	, HSG D	
_	0.	222	80 >75	% Grass c	over, Good	, HSG D
	0.	262	83 Weig	ghted Aver	rage	
	0.	222	-	3% Pervio		
	0.	040	15.2	7% Imperv	vious Area	
	Та	Longeth	Clana	Volgeity	Canaaitu	Description
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
_	· /		. ,	. ,	(05)	
	8.1	88	0.0235	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.34"
	0.1	39	0.2820	8.55		Shallow Concentrated Flow,
						Unpaved Kv= 16.1 fps
	82	127	Total			

8.2 127 Total

#### Subcatchment 9S: P2



### Summary for Subcatchment 11S: P3

Runoff = 35.78 cfs @ 12.15 hrs, Volume= 3.258 af, Depth> 7.26"

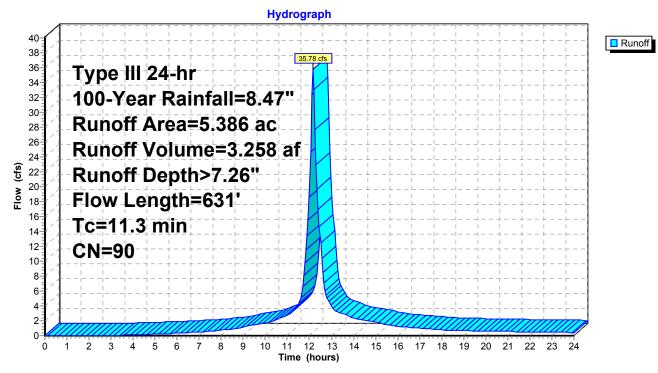
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.47"

Area	(ac) C	N Desc	cription		
3.	.740 9	8 Pave	ed parking	, HSG D	
0.	.000 9	6 Grav	el surface	, HSG D	
0.	.106 7	'4 >75°	% Grass co	over, Good	, HSG C
0.	.256 8	30 >75%	% Grass co	over, Good	, HSG D
0.	.138 7	'9 Woo	ds/grass d	comb., Goo	d, HSG D
1.	.146 7	'0 Woo	ds, Good,	HSG C	
5.	.386 9	0 Weig	ghted Aver	age	
1.	.646	30.5	6% Pervio	us Area	
3.	.740	69.4	4% Imperv	ious Area	
Тс	Length	Slope	Velocity		Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
6.2	100	0.4200	0.27		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.34"
0.1	19	0.4700	3.43		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	27	0.0376	3.12		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.5	65	0.0153	1.99		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
1.0	100	0.0100	1.61		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
1.8	143	0.0070	1.35		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.6	70	0.0143	1.93		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
1.0	107	0.0079	1.80		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
11.3	631	Total			

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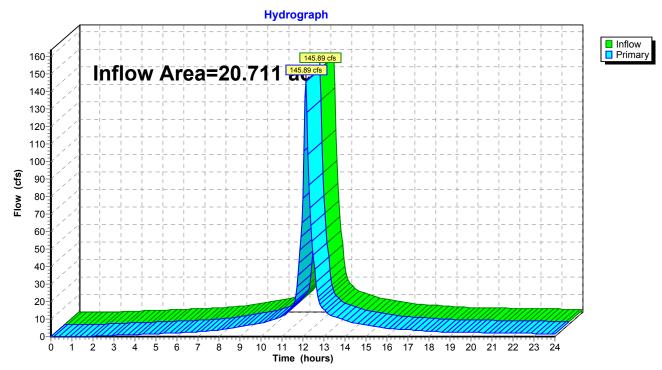




## Summary for Link 8L: PDP1

Inflow Are	ea =	20.711 ac, 85.36% Impervious, Inflow Depth > 7.84" for 100-Year event
Inflow	=	145.89 cfs @ 12.14 hrs, Volume= 13.534 af
Primary	=	145.89 cfs @ 12.14 hrs, Volume= 13.534 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

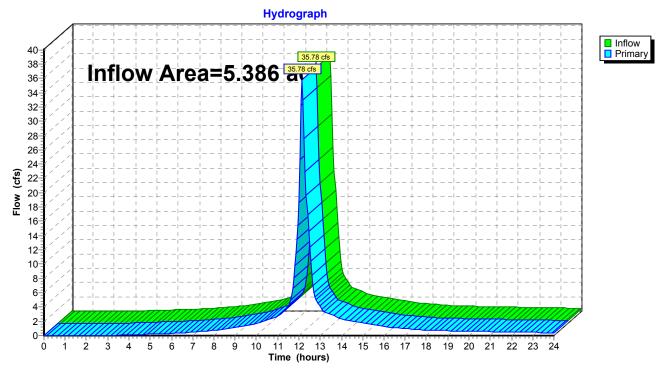


#### Link 8L: PDP1

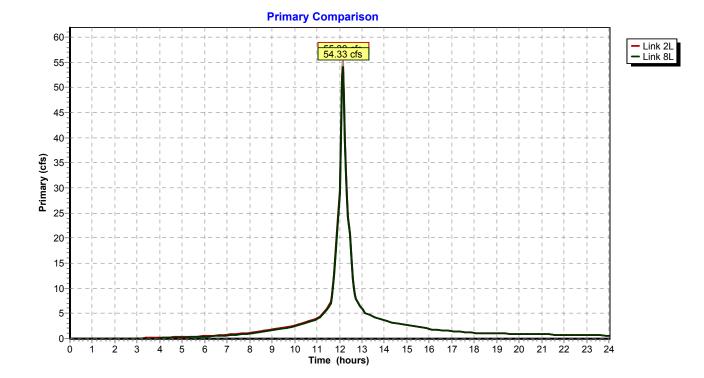
## Summary for Link 12L: PDP2

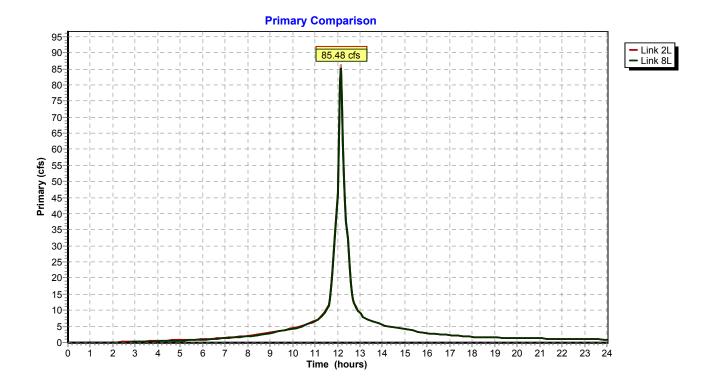
Inflow Are	a =	5.386 ac, 69.44% Impervious, Inflow Depth > 7.26" for 100-Year eve	ent
Inflow	=	35.78 cfs @ 12.15 hrs, Volume= 3.258 af	
Primary	=	35.78 cfs @ 12.15 hrs, Volume= 3.258 af, Atten= 0%, Lag= 0.0	min

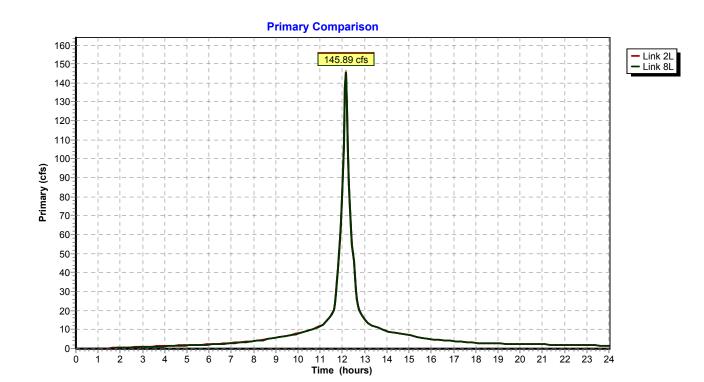
Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.05 hrs

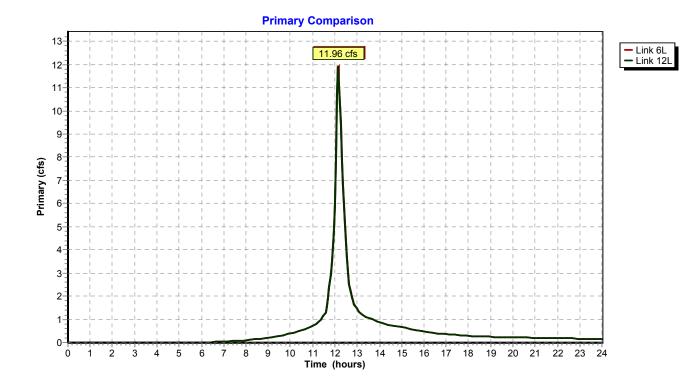


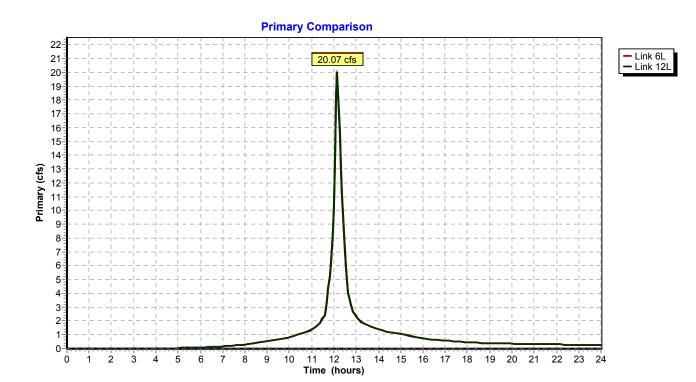
#### Link 12L: PDP2



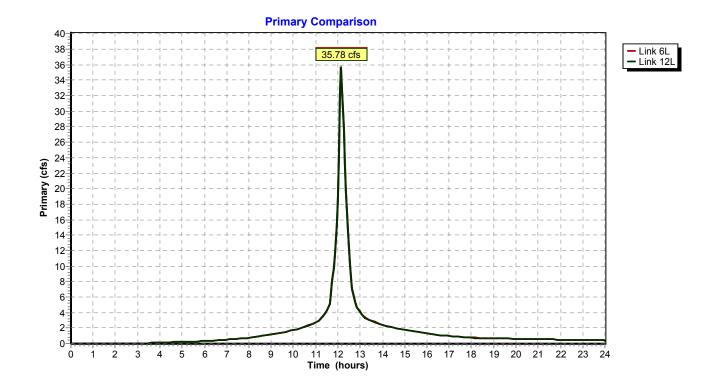




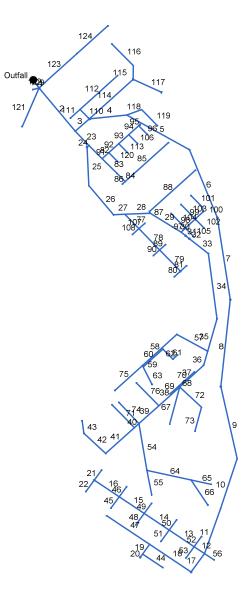








## Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Line		Align	ment			Flow	v Data					Physica	l Data				Line ID
No.	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	34	56	МН	0.00	0.00	0.00	6.0	23.04	1.69	23.61	36	Cir	0.013	1.00	35.03	P-73
2	1	190	-5	DrGrt	0.00	0.08	0.74	6.0	23.78	1.61	26.83	36	Cir	0.013	1.90	39.01	P-72
3	2	63	-93	DrGrt	0.00	0.95	0.81	6.0	26.83	1.01	27.46	30	Cir	0.013	1.82	34.90	P-71
4	3	124	36	DrGrt	0.00	0.20	0.90	6.0	27.56	1.00	28.80	30	Cir	0.013	1.03	37.10	P-79
5	4	238	36	DrGrt	0.00	0.12	0.87	6.0	28.80	1.00	31.18	30	Cir	0.013	1.01	41.00	P-78
6	5	250	39	мн	0.00	0.00	0.00	6.0	31.18	0.78	33.14	30	Cir	0.013	0.26	41.10	P-148
7	6	285	13	мн	0.00	0.00	0.00	6.0	33.14	0.79	35.39	30	Cir	0.013	0.30	45.21	P-154
8	7	300	15	мн	0.00	0.00	0.00	6.0	35.39	0.79	37.76	30	Cir	0.013	0.36	45.84	P-153
9	8	253	-18	DrGrt	0.00	0.70	0.90	6.0	37.76	0.80	39.78	30	Cir	0.013	0.83	45.05	P-152 (1)
10	9	181	30	DrGrt	0.00	0.46	0.90	6.0	39.78	0.75	41.13	30	Cir	0.013	0.50	46.23	P-152
11	10	163	1	DrGrt	0.00	0.65	0.81	6.0	41.13	0.75	42.35	30	Cir	0.013	2.18	47.38	P-151 (1)
12	11	41	106	None	0.00	0.00	0.00	6.0	42.45	0.75	42.76	18	Cir	0.012	1.00	48.20	P-158 (4)
13	12	100	0	None	0.00	0.00	0.00	6.0	42.76	0.76	43.52	18	Cir	0.012	1.00	48.10	P-158 (3)
14	13	100	0	None	0.00	0.00	0.00	6.0	43.52	0.76	44.28	18	Cir	0.012	1.00	48.65	P-158 (2)
15	14	100	0	None	0.00	0.00	0.00	6.0	44.28	0.76	45.04	18	Cir	0.012	1.00	49.00	P-158 (1)
16	15	100	0	мн	0.00	0.00	0.00	6.0	45.04	0.76	45.80	18	Cir	0.012	1.00	49.20	P-158
17	11	77	16	мн	0.00	0.00	0.00	6.0	42.35	0.75	42.93	30	Cir	0.012	1.00	48.76	P-151 (2)
18	17	166	90	DrGrt	0.00	0.50	0.81	6.0	42.93	0.75	44.18	24	Cir	0.012	1.50	48.31	P-151
19	18	41	-90	None	0.00	0.00	0.00	6.0	44.28	1.99	45.09	24	Cir	0.012	1.00	49.38	P-159
20	19	13	0	мн	0.00	0.11	0.95	6.0	45.09	2.02	45.36	24	Cir	0.012	1.00	49.58	P-159 (1)
21	16	42	90	мн	0.00	0.07	0.95	6.0	45.80	0.75	46.12	12	Cir	0.012	1.00	48.75	P-198
22	16	48	-90	мн	0.00	0.07	0.95	6.0	45.80	0.76	46.16	12	Cir	0.012	1.00	49.75	P-216
23	2	67	-3	DrGrt	0.00	0.19	0.90	6.0	26.73	3.08	28.78	36	Cir	0.013	1.60	38.45	E-22
Projec	 t File: 190	826_PipeSi	zing.stm									Number	of lines: 12	4		Date: 8	/26/2019
1 10,00			Ling.outi									Number	or intog. 12				Storm Sawa

		Align	ment			Flow	v Data					Physica	l Data				Line ID
lo.	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
24	23	8	88	мн	0.00	0.00	0.00	6.0	28.88	0.88	28.95	36	Cir	0.013	0.79	38.62	P-65
25	24	133	-49	DrGrt	0.00	0.49	0.81	6.0	29.05	0.56	29.79	36	Cir	0.013	0.97	38.80	P-64
26	25	129	-37	DrGrt	0.00	0.73	0.87	6.0	29.89	0.57	30.62	36	Cir	0.013	1.25	39.50	P-38
27	26	37	-53	None	0.00	0.00	0.00	6.0	30.72	0.62	30.95	36	Cir	0.013	0.80	40.10	P-47 (1)
28	27	84	0	DrGrt	0.00	0.21	0.87	6.0	30.95	0.60	31.45	36	Cir	0.013	1.50	40.80	P-47
29	28	88	35	DrGrt	0.00	0.13	0.87	6.0	31.45	0.53	31.91	36	Cir	0.013	0.50	40.80	P-63
30	29	33	0	DrGrt	0.00	0.49	0.87	6.0	31.91	0.52	32.08	36	Cir	0.013	1.47	41.00	P-122
31	30	19	8	мн	0.00	0.00	0.00	6.0	32.48	1.12	32.69	36	Cir	0.013	0.15	41.10	E-17
32	31	16	0	мн	0.00	0.00	0.00	6.0	32.84	0.99	33.00	36	Cir	0.013	0.15	41.41	E-16
33	32	81	-1	DrGrt	0.00	0.60	0.86	6.0	33.00	1.66	34.35	36	Cir	0.013	1.10	42.18	E-15
34	33	226	43	DrGrt	0.00	0.08	0.90	6.0	34.40	0.43	35.37	30	Cir	0.013	0.66	45.58	E-14
35	34	116	23	DrGrt	0.00	0.33	0.81	6.0	35.47	0.67	36.24	30	Cir	0.013	1.50	45.49	E-13 (1)
36	35	50	0	DrGrt	0.00	0.28	0.87	6.0	36.24	0.68	36.58	30	Cir	0.013	0.86	45.22	E-13
37	36	109	31	DrGrt	0.00	0.00	0.90	6.0	37.13	0.52	37.70	30	Cir	0.013	2.25	45.88	E-12
38	37	94	1	None	0.00	0.00	0.00	6.0	37.90	0.99	38.83	30	Cir	0.013	1.00	46.07	E-11 (1)
39	38	90	0	DrGrt	0.00	0.00	0.90	6.0	38.83	0.99	39.72	30	Cir	0.013	1.50	45.67	E-11
40	39	20	0	None	0.00	0.00	0.00	6.0	40.07	2.11	40.50	24	Cir	0.013	1.00	47.05	E-10 (1)
41	40	130	0	DrGrt	0.00	0.77	0.74	6.0	40.50	2.09	43.21	24	Cir	0.013	1.50	47.70	E-10
42	41	99	87	DrGrt	0.00	0.67	0.78	6.0	43.65	1.50	45.14	18	Cir	0.013	1.00	48.71	E-47
43	42	44	38	DrGrt	0.00	0.05	0.90	6.0	45.30	0.05	45.32	12	Cir	0.013	1.00	47.57	E-45
44	19	80	-90	мн	0.00	0.11	0.95	6.0	45.09	1.50	46.30	12	Cir	0.012	1.00	49.34	P-195
45	15	48	-90	мн	0.00	0.11	0.95	6.0	45.04	0.76	45.40	12	Cir	0.012	1.00	49.27	P-214
46	15	42	90	МН	0.00	0.11	0.95	6.0	45.04	0.75	45.36	12	Cir	0.012	1.00	48.78	P-200
Projec	 t File: 190		 zing.stm									Number	of lines: 12	4		Date: 8	)/26/2019

	Dnstr Line No.	Line Length (ft)	Defl angle	Junc	Known	1											1
48 49 50	18		(deg)	Туре	Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	-
49 50		172	0	DrGrt	0.00	0.61	0.72	6.0	44.18	0.75	45.47	24	Cir	0.012	1.00	49.39	P-156
50	14	48	-90	мн	0.00	0.11	0.95	6.0	44.28	0.80	44.66	12	Cir	0.012	1.00	49.25	P-212
	14	42	90	мн	0.00	0.11	0.95	6.0	44.28	0.80	44.62	12	Cir	0.012	1.00	48.10	P-202
51	13	42	90	мн	0.00	0.11	0.95	6.0	43.52	0.80	43.86	12	Cir	0.012	1.00	47.85	P-204
51	13	48	-90	мн	0.00	0.11	0.95	6.0	43.52	0.76	43.88	12	Cir	0.012	1.00	48.88	P-210
52	12	42	90	мн	0.00	0.11	0.95	6.0	42.76	0.80	43.10	12	Cir	0.012	1.00	47.71	P-206
53	12	48	-90	мн	0.00	0.11	0.95	6.0	42.76	0.80	43.14	12	Cir	0.012	1.00	48.50	P-208
54	39	163	-56	DrGrt	0.00	0.02	0.90	6.0	39.82	0.39	40.46	30	Cir	0.013	1.41	45.96	E-39
55	54	86	-1	DrGrt	0.00	0.28	0.90	6.0	42.11	1.03	42.99	24	Cir	0.013	1.00	47.76	E-43
56	11	38	-74	DrGrt	0.00	0.51	0.60	6.0	42.45	0.76	42.74	18	Cir	0.012	1.00	45.10	P-217
57	35	117	104	DrGrt	0.00	0.18	0.81	6.0	36.24	1.00	37.41	30	Cir	0.013	1.45	45.10	P-25
58	57	156	-73	DrGrt	0.00	0.10	0.60	6.0	37.51	1.00	39.07	30	Cir	0.013	2.23	45.10	P-24
59	58	5	-90	мн	0.00	0.00	0.95	6.0	40.89	1.00	40.94	8	Cir	0.012	1.00	45.98	P-117
60	59	88	-90	мн	0.00	0.00	0.00	6.0	40.94	1.00	41.82	8	Cir	0.012	1.00	45.99	P-116
61	60	48	90	мн	0.00	0.12	0.95	6.0	41.82	0.99	42.29	8	Cir	0.012	1.00	46.00	P-115
62	61	21	-90	мн	0.00	0.02	0.95	6.0	42.29	1.00	42.50	8	Cir	0.012	1.00	46.00	P-114
63	58	72	-73	DrGrt	0.00	0.38	0.90	6.0	41.43	1.00	42.15	18	Cir	0.013	1.00	45.43	P-82
64	54	149	-68	DrGrt	0.00	0.39	0.90	6.0	40.66	0.22	40.99	30	Cir	0.013	1.12	46.36	E-40
65	64	66	0	DrGrt	0.00	0.06	0.90	6.0	41.04	0.92	41.65	24	Cir	0.013	1.00	45.40	E-41
66	64	102	45	DrGrt	0.00	0.06	0.90	6.0	40.99	0.62	41.62	24	Cir	0.013	1.00	44.87	P-147
67	37	132	-32	DrGrt	0.00	0.65	0.90	6.0	39.93	0.71	40.87	18	Cir	0.013	1.00	45.57	E-37
68	37	5	90	мн	0.00	0.00	0.00	6.0	41.25	1.03	41.30	8	Cir	0.012	1.00	45.94	P-108
69	68	39	-90	мн	0.00	0.08	0.95	6.0	41.30	1.01	41.69	8	Cir	0.012	1.00	45.94	P-109
Project F	 File: 190	826_PipeSi	 zing.stm									Number	of lines: 124	4		Date: 8	/26/2019

Line		Align	ment			Flov	v Data					Physica	l Data				Line ID
No.	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
70	68	70	90	МН	0.00	0.06	0.95	6.0	41.30	0.99	42.00	8	Cir	0.012	1.00	45.97	P-107
71	40	108	90	мн	0.00	0.22	0.95	6.0	40.50	2.00	42.65	12	Cir	0.012	1.00	47.18	P-193
72	37	100	-92	DrGrt	0.00	1.23	0.87	6.0	37.86	1.82	39.67	24	Cir	0.013	1.34	44.37	E-35
73	72	84	60	DrGrt	0.00	0.44	0.90	6.0	40.07	0.69	40.65	18	Cir	0.013	1.00	45.28	E-36
74	39	99	91	DrGrt	0.00	0.01	0.90	6.0	40.07	2.16	42.20	18	Cir	0.013	1.00	46.50	E-38
75	58	124	0	DrGrt	0.00	0.24	0.75	6.0	39.17	1.00	40.41	15	Cir	0.012	1.00	46.70	P-52
76	38	108	90	мн	0.00	0.22	0.95	6.0	38.83	2.00	40.99	12	Cir	0.012	1.00	45.98	P-191
77	27	67	49	None	0.00	0.00	0.00	6.0	30.95	2.25	32.47	24	Cir	0.012	1.00	41.07	P-160 (2)
78	77	100	0	None	0.00	0.00	0.00	6.0	32.47	2.26	34.73	24	Cir	0.012	1.00	41.65	P-160 (1)
79	78	100	0	мн	0.00	0.00	0.00	6.0	34.73	2.26	36.99	24	Cir	0.012	1.00	41.93	P-160
80	79	28	90	мн	0.00	0.22	0.95	6.0	36.99	2.24	37.61	12	Cir	0.012	1.00	43.59	P-180
81	79	27	-90	мн	0.00	0.22	0.95	6.0	36.99	2.23	37.60	12	Cir	0.012	1.00	42.83	P-174
82	23	63	-2	None	0.00	0.00	0.00	6.0	28.83	0.13	28.91	36	Cir	0.013	1.00	38.84	E-21 (3)
83	82	70	0	DrGrt	0.00	0.00	0.00	6.0	28.91	0.11	28.99	36	Cir	0.013	0.50	40.13	E-21
84	83	39	1	мн	0.00	0.00	0.90	6.0	34.66	0.99	35.05	12	Cir	0.012	1.00	40.87	P-91
85	84	195	-90	мн	0.00	0.26	0.95	6.0	35.05	1.00	37.00	12	Cir	0.012	1.00	40.91	P-90
86	84	12	90	мн	0.00	0.03	0.95	6.0	35.05	1.02	35.17	12	Cir	0.012	1.00	40.77	P-92
87	28	6	-110	мн	0.00	0.00	0.00	6.0	34.87	0.94	34.93	12	Cir	0.012	0.95	40.90	P-100
88	87	207	69	мн	0.00	0.29	0.95	6.0	34.93	1.00	37.00	12	Cir	0.012	1.00	40.92	P-99
89	78	27	-90	мн	0.00	0.22	0.95	6.0	34.56	2.26	35.17	12	Cir	0.012	1.00	41.88	P-176
90	78	28	90	мн	0.00	0.22	0.95	6.0	34.56	2.25	35.19	12	Cir	0.012	1.00	42.33	P-182
91	82	30	-89	None	0.00	0.00	0.00	6.0	28.91	2.99	29.81	18	Cir	0.012	1.00	39.35	P-162 (4)
92	91	45	0	None	0.00	0.00	0.00	6.0	29.81	3.00	31.16	18	Cir	0.012	1.00	39.51	P-162 (3)
Projec	t File: 190	826_PipeSi	izing.stm									Number	of lines: 12	4		Date: 8	/26/2019
Projec	t File: 190	826_PipeSi	izing.stm									Number	of lines: 12	4		Date: 8	/26/2019 Storm Sewe

93	Dnstr Line No. 92 93	Line Length (ft) 45	Defl angle (deg)	Junc Type	Known Q	Drng	Runoff										
		45			(cfs)	Area (ac)	Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
94	93		0	None	0.00	0.00	0.00	6.0	31.16	3.00	32.51	18	Cir	0.012	1.00	37.91	P-162 (2)
		45	0	None	0.00	0.00	0.00	6.0	32.51	3.00	33.86	18	Cir	0.012	1.00	37.06	P-162 (1)
95	94	5	0	мн	0.00	0.02	0.95	6.0	33.86	3.00	34.01	18	Cir	0.012	1.00	38.54	P-162
96	94	42	90	мн	0.00	0.20	0.95	6.0	33.86	3.00	35.11	12	Cir	0.012	1.00	39.23	P-166
97	30	19	-77	None	0.00	0.00	0.00	6.0	32.58	0.83	32.74	24	Cir	0.013	1.00	39.68	E-30 (1) (1)
98	97	45	0	None	0.00	0.00	0.00	6.0	32.74	0.84	33.12	24	Cir	0.013	1.00	39.60	E-30 (1)
99	98	35	0	DrGrt	0.00	0.67	0.86	6.0	33.12	0.83	33.41	24	Cir	0.013	1.21	40.51	E-30
100	99	16	-51	None	0.00	0.00	0.00	6.0	33.34	3.00	33.82	12	Cir	0.012	0.67	40.75	P-186
101	100	56	-38	мн	0.00	0.22	0.95	6.0	33.99	3.00	35.66	12	Cir	0.012	1.00	40.98	P-185
102	98	41	91	мн	0.00	0.22	0.95	6.0	32.95	2.99	34.19	12	Cir	0.012	1.00	41.32	P-189
103	98	69	-89	мн	0.00	0.22	0.95	6.0	32.95	3.00	35.01	12	Cir	0.012	1.00	40.99	P-187
104	97	69	-89	мн	0.00	0.22	0.95	6.0	32.57	3.01	34.65	12	Cir	0.012	1.00	40.99	P-188
105	97	41	91	мн	0.00	0.22	0.95	6.0	32.57	2.99	33.79	12	Cir	0.012	1.00	41.83	P-190
106	93	42	90	мн	0.00	0.22	0.95	6.0	32.51	3.00	33.76	12	Cir	0.012	1.00	39.46	P-167
107	77	27	-90	мн	0.00	0.22	0.95	6.0	32.30	2.26	32.90	12	Cir	0.012	1.00	41.08	P-178
108	77	28	90	мн	0.00	0.22	0.95	6.0	32.30	2.25	32.94	12	Cir	0.012	1.00	41.02	P-184
109	1	21	108	DrGrt	0.00	0.23	0.74	6.0	32.00	0.99	32.21	12	Cir	0.013	1.00	35.00	P-155
110	3	44	-91	мн	0.00	0.00	0.00	6.0	30.87	0.99	31.31	12	Cir	0.012	1.00	36.93	P-84
111	110	44	-90	мн	0.00	0.08	0.95	6.0	31.31	1.01	31.75	12	Cir	0.012	1.00	36.90	P-85
112	110	169	90	мн	0.00	0.25	0.95	6.0	31.31	1.00	33.00	12	Cir	0.012	1.00	36.93	P-83
113	92	42	90	мн	0.00	0.22	0.95	6.0	31.16	3.00	32.41	12	Cir	0.012	1.00	40.01	P-169
114	3	198	-1	DrGrt	0.00	0.48	0.81	6.0	27.56	1.04	29.62	24	Cir	0.013	1.42	35.00	P-70
115	114	47	-47	DrGrt	0.00	0.21	0.81	6.0	29.70	0.91	30.13	18	Cir	0.013	1.10	34.90	P-68
Project F	- ile: 190	826_PipeSi	 izing.stm									Number	of lines: 124	 1		Date: 8	/26/2019

Line		Align	ment			Flow	Data					Physical	Data				Line ID
No.	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
116	115	103	-43	DrGrt	0.00	0.02	0.90	6.0	30.21	0.94	31.18	18	Cir	0.013	1.00	35.02	P-34
117	114	103	69	DrGrt	0.00	0.25	0.87	6.0	30.07	2.00	32.14	15	Cir	0.013	1.00	0.00	P-80
118	4	52	-15	None	0.00	0.00	0.00	6.0	28.90	2.00	29.95	12	Cir	0.013	0.94	36.39	P-164
119	118	75	68	МН	0.00	0.22	0.95	6.0	29.95	2.00	31.45	12	Cir	0.013	1.00	39.65	P-163
120	91	42	90	МН	0.00	0.22	0.95	6.0	29.81	3.00	31.06	12	Cir	0.012	1.00	39.69	P-172
121	1	143	56	DrGrt	0.00	0.32	0.90	6.0	29.41	1.50	31.56	18	Cir	0.013	1.00	37.00	P-42
122	1	28	-101	DrGrt	0.00	0.11	0.81	6.0	27.53	0.85	27.77	18	Cir	0.013	0.50	34.70	P-76
123	122	144	1	DrGrt	0.00	0.22	0.81	6.0	27.85	0.99	29.27	18	Cir	0.013	0.50	34.35	P-75
124	123	139	0	DrGrt	0.00	0.15	0.81	6.0	29.35	0.90	30.60	18	Cir	0.013	1.00	33.65	P-74
Project	File: 190	826_PipeSi	zing.stm						-			Number o	of lines: 124	4		Date: 8	/26/2019

Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type	
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)			
1	P-73	86.63	52.00	0.00	0.00	0.00	36	Cir	34	23.04	23.61	1.69	25.79	25.95	n/a	25.95	Outfall	МН	
2	P-72	84.56	49.94	0.74	7.49	0.08	36	Cir	190	23.78	26.83	1.61	25.95	29.13	n/a	29.13	1	Dp-Grate	
3	P-71	41.12	23.51	0.81	7.49	0.95	30	Cir	63	26.83	27.46	1.01	29.13	29.11 j	n/a	29.11	2	Dp-Grate	
4	P-79	41.01	21.69	0.90	7.49	0.20	30	Cir	124	27.56	28.80	1.00	29.11	30.38	n/a	30.38	3	Dp-Grate	
5	P-78	41.04	20.25	0.87	7.49	0.12	30	Cir	238	28.80	31.18	1.00	30.38	32.71 j	n/a	32.71	4	Dp-Grate	
6	P-148	36.28	20.24	0.00	0.00	0.00	30	Cir	250	31.18	33.14	0.78	32.71	34.67	n/a	34.67	5	мн	
7	P-154	36.42	20.83	0.00	0.00	0.00	30	Cir	285	33.14	35.39	0.79	34.67	36.94	0.20	36.94	6	мн	
8	P-153	36.45	21.46	0.00	0.00	0.00	30	Cir	300	35.39	37.76	0.79	36.94	39.33	n/a	39.33	7	МН	
9	P-152 (1)	36.66	21.99	0.90	7.49	0.70	30	Cir	253	37.76	39.78	0.80	39.33	41.37	0.57	41.37	8	Dp-Grate	
10	P-152	35.45	18.82	0.90	7.49	0.46	30	Cir	181	39.78	41.13	0.75	41.37	42.60 j	n/a	42.60	9	Dp-Grate	
11	P-151 (1)	35.51	16.79	0.81	7.49	0.65	30	Cir	163	41.13	42.35	0.75	42.60	43.73 j	n/a	43.73	10	Dp-Grate	
12	P-158 (4)	9.84	5.73	0.00	0.00	0.00	18	Cir	41	42.45	42.76	0.75	43.73	43.68	n/a	43.68	11	None	
13	P-158 (3)	9.92	4.57	0.00	0.00	0.00	18	Cir	100	42.76	43.52	0.76	43.68	44.34 j	n/a	44.34	12	None	
14	P-158 (2)	9.92	3.40	0.00	0.00	0.00	18	Cir	100	43.52	44.28	0.76	44.34	44.98 j	n/a	44.98	13	None	
15	P-158 (1)	9.92	2.19	0.00	0.00	0.00	18	Cir	100	44.28	45.04	0.76	44.98	45.60 j	n/a	45.60	14	None	
16	P-158	9.92	0.95	0.00	0.00	0.00	18	Cir	100	45.04	45.80	0.76	45.60	46.16 j	n/a	46.16	15	MH	
17	P-151 (2)	38.60	6.84	0.00	0.00	0.00	30	Cir	77	42.35	42.93	0.75	43.73	43.80 j	n/a	43.80	11	MH	
18	P-151	21.26	7.11	0.81	7.49	0.50	24	Cir	166	42.93	44.18	0.75	43.80	45.13	n/a	45.13	17	Dp-Grate	
19	P-159	34.60	1.49	0.00	0.00	0.00	24	Cir	41	44.28	45.09	1.99	45.13	45.51 j	n/a	45.51	18	None	
20	P-159 (1)	34.83	0.78	0.95	7.49	0.11	24	Cir	13	45.09	45.36	2.02	45.51	45.66 j	n/a	45.66	19	MH	
21	P-198	3.35	0.50	0.95	7.49	0.07	12	Cir	42	45.80	46.12	0.75	46.16	46.41 j	n/a	46.41	16	MH	
22	P-216	3.36	0.50	0.95	7.49	0.07	12	Cir	48	45.80	46.16	0.76	46.16	46.45 j	n/a	46.45	16	MH	
23	E-22	117.06	33.32	0.90	7.49	0.19	36	Cir	67	26.73	28.78	3.08	29.13	30.65 j	n/a	30.65	2	Dp-Grate	
Projec	t File: 190826_	_ PipeSizir	ig.stm	1	1	<u> </u>		L	1		<u>I</u>	1	Num	ber of lines	s: 124		Da	te: 8/26/2019	
NOTE	S: Intensity = 1	82.59 / (In	let time +	+ 19.10) ^	0.99 R	eturn pe	riod = 25	5 Yrs.;	** Critica	l depth									

Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type	
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)			
24	P-65	62.39	29.94	0.00	0.00	0.00	36	Cir	8	28.88	28.95	0.88	30.65	30.72 j	n/a	30.72	23	мн	
25	P-64	49.84	30.04	0.81	7.49	0.49	36	Cir	133	29.05	29.79	0.56	30.73	31.56	n/a	31.56	24	Dp-Grate	
26	P-38	50.20	29.09	0.87	7.49	0.73	36	Cir	129	29.89	30.62	0.57	31.56	32.36	0.90	32.36	25	Dp-Grate	
27	P-47 (1)	52.50	27.43	0.00	0.00	0.00	36	Cir	37	30.72	30.95	0.62	32.36	32.64	n/a	32.64	26	None	
28	P-47	51.51	24.16	0.87	7.49	0.21	36	Cir	84	30.95	31.45	0.60	32.64	33.03 j	n/a	33.03	27	Dp-Grate	
29	P-63	48.35	23.00	0.87	7.49	0.13	36	Cir	88	31.45	31.91	0.53	33.03	33.45	n/a	33.45	28	Dp-Grate	
30	P-122	47.94	22.72	0.87	7.49	0.49	36	Cir	33	31.91	32.08	0.52	33.45	33.61 j	n/a	33.61	29	Dp-Grate	
31	E-17	70.66	17.25	0.00	0.00	0.00	36	Cir	19	32.48	32.69	1.12	33.61	34.02	n/a	34.02	30	мн	
32	E-16	66.42	17.27	0.00	0.00	0.00	36	Cir	16	32.84	33.00	0.99	34.02	34.33	0.08	34.33	31	мн	
33	E-15	86.05	17.32	0.86	7.49	0.60	36	Cir	81	33.00	34.35	1.66	34.33	35.68	0.56	35.68	32	Dp-Grate	
34	E-14	26.89	16.05	0.90	7.49	0.08	30	Cir	226	34.40	35.37	0.43	35.79	36.76	0.34	37.10	33	Dp-Grate	
35	E-13 (1)	33.48	15.91	0.81	7.49	0.33	30	Cir	116	35.47	36.24	0.67	37.10	37.59 j	n/a	37.59	34	Dp-Grate	
36	E-13	33.90	12.87	0.87	7.49	0.28	30	Cir	50	36.24	36.58	0.68	37.59	37.78 j	n/a	37.78	35	Dp-Grate	
37	E-12	29.68	12.25	0.90	0.00	0.00	30	Cir	109	37.13	37.70	0.52	38.25	38.87	n/a	38.87	36	Dp-Grate	
38	E-11 (1)	40.88	6.32	0.00	0.00	0.00	30	Cir	94	37.90	38.83	0.99	38.87	39.66 j	n/a	39.66	37	None	
39	E-11	40.88	5.79	0.90	0.00	0.00	30	Cir	90	38.83	39.72	0.99	39.66	40.51 j	n/a	40.51	38	Dp-Grate	
40	E-10 (1)	32.83	8.94	0.00	0.00	0.00	24	Cir	20	40.07	40.50	2.11	40.78	41.57	0.43	41.57	39	None	
41	E-10	32.68	7.78	0.74	7.49	0.77	24	Cir	130	40.50	43.21	2.09	41.57	44.20 j	n/a	44.20	40	Dp-Grate	
42	E-47	12.86	3.98	0.78	7.49	0.67	18	Cir	99	43.65	45.14	1.50	44.22	45.90	n/a	45.90	41	Dp-Grate	
43	E-45	0.76	0.34	0.90	7.49	0.05	12	Cir	44	45.30	45.32	0.05	45.90	45.91	0.01	45.92	42	Dp-Grate	
44	P-195	4.73	0.78	0.95	7.49	0.11	12	Cir	80	45.09	46.30	1.50	45.51	46.67 j	n/a	46.67	19	МН	
45	P-214	3.36	0.78	0.95	7.49	0.11	12	Cir	48	45.04	45.40	0.76	45.60	45.77 j	n/a	45.77	15	МН	
46	P-200	3.35	0.78	0.95	7.49	0.11	12	Cir	42	45.04	45.36	0.75	45.60	45.73 j	n/a	45.73	15	MH	
Projec	t File: 190826_	_PipeSizir	ig.stm	<u> </u>	1	1	<u> </u>	L	1	L	1	1	Num	ber of lines	s: 124	L	Da	ate: 8/26/201	9
NOTE	S: Intensity = 1	82.59 / (In	let time +	- 19.10) ^	0.99 R	leturn pe	riod = 25	5 Yrs.;	** Critica	l depth							<b>!</b>		

Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type	
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)			
47	P-156	21.19	3.29	0.72	7.49	0.61	24	Cir	172	44.18	45.47	0.75	45.13	46.10 j	n/a	46.10	18	Dp-Grate	
48	P-212	3.45	0.78	0.95	7.49	0.11	12	Cir	48	44.28	44.66	0.80	44.98	45.03 j	n/a	45.03	14	мн	
49	P-202	3.45	0.78	0.95	7.49	0.11	12	Cir	42	44.28	44.62	0.80	44.98	44.99 j	n/a	44.99	14	мн	
50	P-204	3.45	0.78	0.95	7.49	0.11	12	Cir	42	43.52	43.86	0.80	44.34	44.23	0.14	44.23	13	мн	
51	P-210	3.36	0.78	0.95	7.49	0.11	12	Cir	48	43.52	43.88	0.76	44.34	44.25	0.14	44.25	13	мн	
52	P-206	3.45	0.78	0.95	7.49	0.11	12	Cir	42	42.76	43.10	0.80	43.68	43.47	0.14	43.47	12	МН	
53	P-208	3.45	0.78	0.95	7.49	0.11	12	Cir	48	42.76	43.14	0.80	43.68	43.51	0.14	43.51	12	мн	
54	E-39	25.71	3.28	0.90	7.49	0.02	30	Cir	163	39.82	40.46	0.39	40.51	41.05	n/a	41.05	39	Dp-Grate	
55	E-43	22.94	1.89	0.90	7.49	0.28	24	Cir	86	42.11	42.99	1.03	42.50	43.47	n/a	43.47	54	Dp-Grate	
56	P-217	9.94	2.29	0.60	7.49	0.51	18	Cir	38	42.45	42.74	0.76	43.73	43.31	0.21	43.31	11	Dp-Grate	
57	P-25	41.05	5.50	0.81	7.49	0.18	30	Cir	117	36.24	37.41	1.00	37.59	38.18 j	n/a	38.18	35	Dp-Grate	
58	P-24	41.07	4.98	0.60	7.49	0.10	30	Cir	156	37.51	39.07	1.00	38.18	39.81	n/a	39.81	57	Dp-Grate	
59	P-117	1.31	0.93	0.95	0.00	0.00	8	Cir	5	40.89	40.94	1.00	41.31	41.40	0.21	41.40	58	МН	
60	P-116	1.31	0.95	0.00	0.00	0.00	8	Cir	88	40.94	41.82	1.00	41.40	42.28	n/a	42.28	59	МН	
61	P-115	1.30	0.96	0.95	7.49	0.12	8	Cir	48	41.82	42.29	0.99	42.28	42.76	0.21	42.76	60	МН	
62	P-114	1.31	0.14	0.95	7.49	0.02	8	Cir	21	42.29	42.50	1.00	42.76	42.67	n/a	42.67	61	МН	
63	P-82	10.50	2.56	0.90	7.49	0.38	18	Cir	72	41.43	42.15	1.00	41.93	42.76	0.23	42.76	58	Dp-Grate	
64	E-40	19.31	2.26	0.90	7.49	0.39	30	Cir	149	40.66	40.99	0.22	41.24	41.57	0.12	41.69	54	Dp-Grate	
65	E-41	21.69	0.40	0.90	7.49	0.06	24	Cir	66	41.04	41.65	0.92	41.69	41.87 j	n/a	41.87	64	Dp-Grate	
66	P-147	17.77	0.40	0.90	7.49	0.06	24	Cir	102	40.99	41.62	0.62	41.69	41.84 j	n/a	41.84	64	Dp-Grate	
67	E-37	8.87	4.38	0.90	7.49	0.65	18	Cir	132	39.93	40.87	0.71	40.67	41.67	0.32	41.67	37	Dp-Grate	
68	P-108	1.33	0.96	0.00	0.00	0.00	8	Cir	5	41.25	41.30	1.03	41.67	41.76	0.21	41.76	37	МН	
69	P-109	1.32	0.57	0.95	7.49	0.08	8	Cir	39	41.30	41.69	1.01	41.76	42.04 j	n/a	42.04	68	МН	
Project File: 190826_PipeSizing.stm     Number of lines: 124     Date: 8/26/2019													9						
NOTE	S: Intensity = 1	82.59 / (ln	let time +	- 19.10) ^ (	0.99 R	eturn pe	riod = 25	SYrs.;	** Critica	l depth									

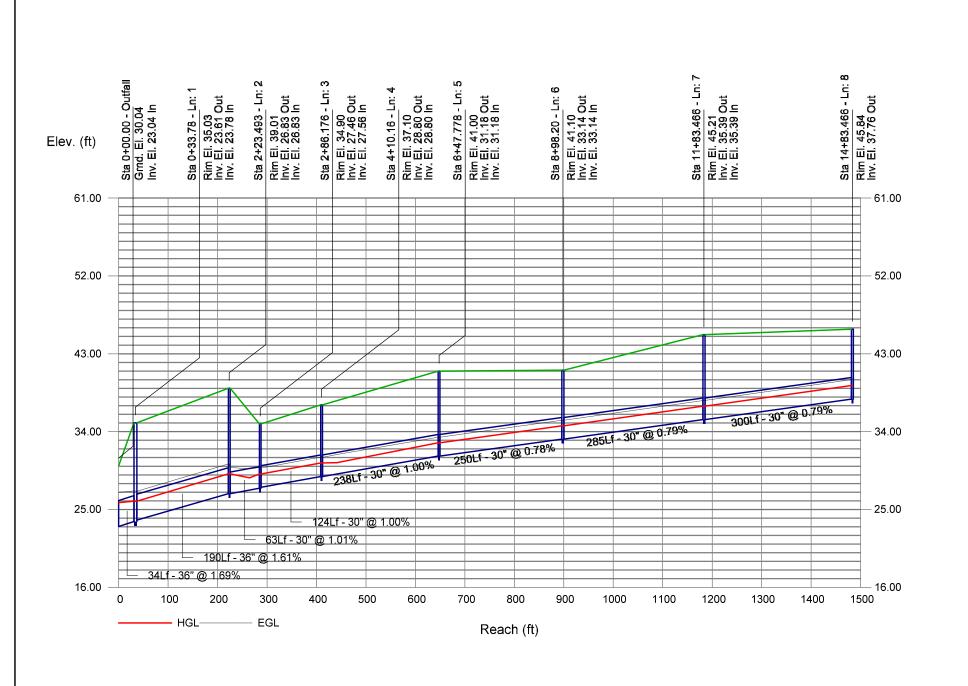
Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type	
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)			
70	P-107	1.30	0.43	0.95	7.49	0.06	8	Cir	70	41.30	42.00	0.99	41.76	42.30 j	n/a	42.30	68	МН	
71	P-193	5.45	1.56	0.95	7.49	0.22	12	Cir	108	40.50	42.65	2.00	41.57	43.18 j	n/a	43.18	40	МН	
72	E-35	30.50	10.63	0.87	7.49	1.23	24	Cir	100	37.86	39.67	1.82	38.87	40.84	0.65	40.84	37	Dp-Grate	
73	E-36	8.75	2.96	0.90	7.49	0.44	18	Cir	84	40.07	40.65	0.69	40.84	41.30 j	n/a	41.30	72	Dp-Grate	
74	E-38	15.42	0.07	0.90	7.49	0.01	18	Cir	99	40.07	42.20	2.16	40.51	42.29 j	n/a	42.29	39	Dp-Grate	
75	P-52	7.00	1.35	0.75	7.49	0.24	15	Cir	124	39.17	40.41	1.00	39.81	40.87 j	n/a	40.87	58	Dp-Grate	
76	P-191	5.46	1.56	0.95	7.49	0.22	12	Cir	108	38.83	40.99	2.00	39.66	41.52 j	n/a	41.52	38	МН	
77	P-160 (2)	36.77	8.46	0.00	0.00	0.00	24	Cir	67	30.95	32.47	2.25	32.64	33.51 j	n/a	33.51	27	None	
78	P-160 (1)	36.84	5.82	0.00	0.00	0.00	24	Cir	100	32.47	34.73	2.26	33.51	35.58 j	n/a	35.58	77	None	
79	P-160	36.82	3.10	0.00	0.00	0.00	24	Cir	100	34.73	36.99	2.26	35.58	37.60 j	n/a	37.60	78	МН	
80	P-180	5.77	1.56	0.95	7.49	0.22	12	Cir	28	36.99	37.61	2.24	37.60	38.14 j	n/a	38.14	79	МН	
81	P-174	5.76	1.56	0.95	7.49	0.22	12	Cir	27	36.99	37.60	2.23	37.60	38.13 j	n/a	38.13	79	МН	
82	E-21 (3)	23.75	6.76	0.00	0.00	0.00	36	Cir	63	28.83	28.91	0.13	30.65	30.67	0.04	30.71	23	None	
83	E-21	22.57	1.94	0.00	0.00	0.00	36	Cir	70	28.91	28.99	0.11	30.71	30.71	0.00	30.71	82	Dp-Grate	
84	P-91	3.85	1.96	0.90	0.00	0.00	12	Cir	39	34.66	35.05	0.99	35.17	35.65	0.25	35.65	83	МН	
85	P-90	3.86	1.85	0.95	7.49	0.26	12	Cir	195	35.05	37.00	1.00	35.65	37.58 j	n/a	37.58	84	мн	
86	P-92	3.90	0.21	0.95	7.49	0.03	12	Cir	12	35.05	35.17	1.02	35.65	35.36	n/a	35.36	84	МН	
87	P-100	3.73	1.96	0.00	0.00	0.00	12	Cir	6	34.87	34.93	0.94	35.38	35.53	n/a	35.53	28	мн	
88	P-99	3.86	2.06	0.95	7.49	0.29	12	Cir	207	34.93	37.00	1.00	35.53	37.61	n/a	37.61	87	МН	
89	P-176	5.80	1.56	0.95	7.49	0.22	12	Cir	27	34.56	35.17	2.26	35.58	35.70 j	n/a	35.70	78	мн	
90	P-182	5.78	1.56	0.95	7.49	0.22	12	Cir	28	34.56	35.19	2.25	35.58	35.72 j	n/a	35.72	78	мн	
91	P-162 (4)	19.67	5.67	0.00	0.00	0.00	18	Cir	30	28.91	29.81	2.99	30.71	30.73 j	n/a	30.73	82	None	
92	P-162 (3)	19.70	4.30	0.00	0.00	0.00	18	Cir	45	29.81	31.16	3.00	30.73	31.95 j	n/a	31.95	91	None	
Projec	t File: 190826_	PipeSizir	ıg.stm					·			L		Num	nber of lines	s: 124		Dε	ate: 8/26/201	I9
NOTE	S: Intensity = 18	82.59 / (lr	ilet time -	+ 19.10) ^	0.99 F	≀eturn pe	riod = 2	5 Yrs. ;	** Critica	l depth									

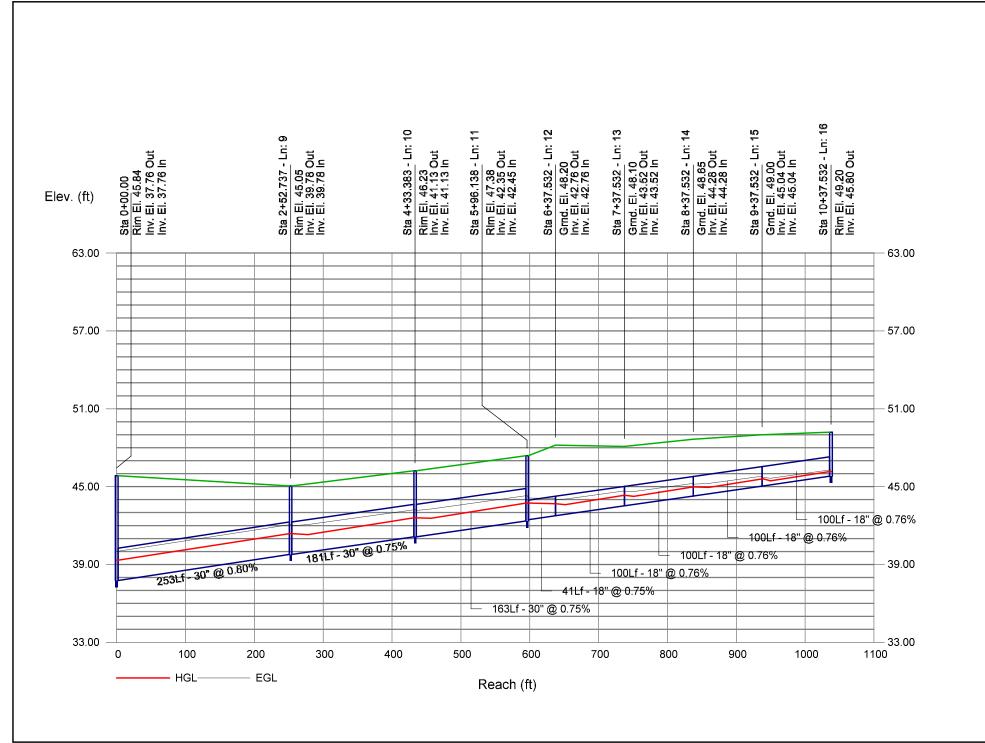
# **MyReport**

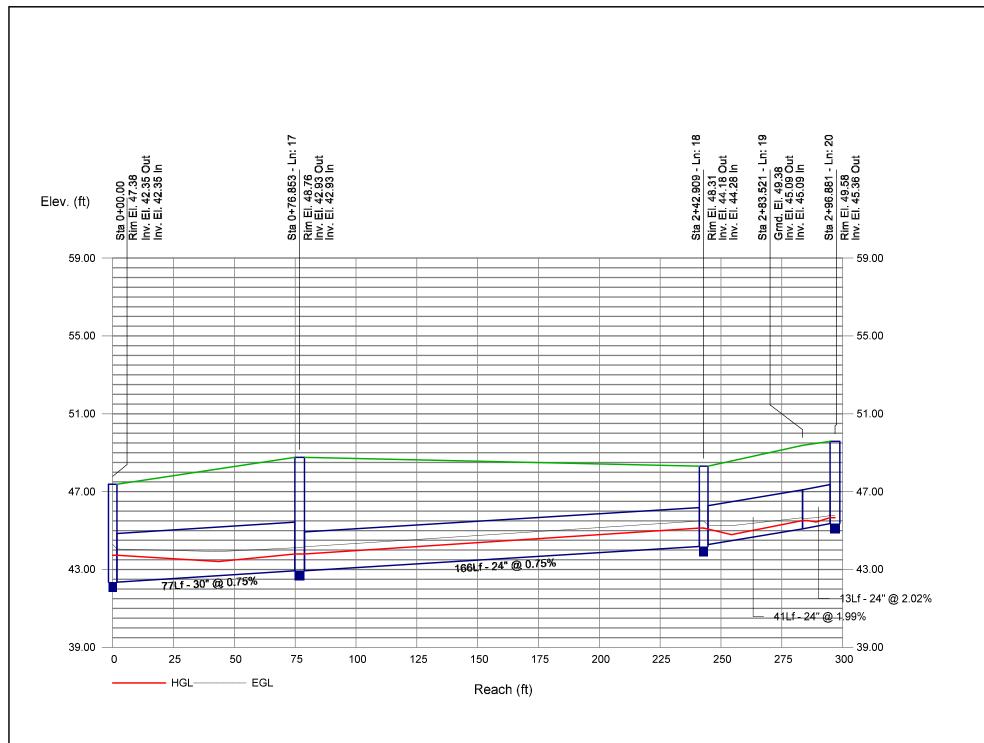
Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type	
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)			
93	P-162 (2)	19.70	2.91	0.00	0.00	0.00	18	Cir	45	31.16	32.51	3.00	31.95	33.16 j	n/a	33.16	92	None	
94	P-162 (1)	19.70	1.50	0.00	0.00	0.00	18	Cir	45	32.51	33.86	3.00	33.16	34.32 j	n/a	34.32	93	None	
95	P-162	19.69	0.14	0.95	7.49	0.02	18	Cir	5	33.86	34.01	3.00	34.32	34.15	n/a	34.15	94	мн	
96	P-166	6.68	1.42	0.95	7.49	0.20	12	Cir	42	33.86	35.11	3.00	34.32	35.61	n/a	35.61	94	мн	
97	E-30 (1) (1)	20.55	11.59	0.00	0.00	0.00	24	Cir	19	32.58	32.74	0.83	33.66	33.96	0.52	33.96	30	None	
98	E-30 (1)	20.79	8.69	0.00	0.00	0.00	24	Cir	45	32.74	33.12	0.84	33.96	34.17 j	n/a	34.17	97	None	
99	E-30	20.57	5.74	0.86	7.49	0.67	24	Cir	35	33.12	33.41	0.83	34.17	34.26 j	n/a	34.26	98	Dp-Grate	
100	P-186	6.68	1.54	0.00	0.00	0.00	12	Cir	16	33.34	33.82	3.00	34.26	34.35 j	n/a	34.35	99	None	
101	P-185	6.69	1.56	0.95	7.49	0.22	12	Cir	56	33.99	35.66	3.00	34.35	36.19	n/a	36.19	100	мн	
102	P-189	6.68	1.56	0.95	7.49	0.22	12	Cir	41	32.95	34.19	2.99	34.17	34.72 j	n/a	34.72	98	мн	
103	P-187	6.69	1.56	0.95	7.49	0.22	12	Cir	69	32.95	35.01	3.00	34.17	35.54 j	n/a	35.54	98	мн	
104	P-188	6.69	1.56	0.95	7.49	0.22	12	Cir	69	32.57	34.65	3.01	33.96	35.18 j	n/a	35.18	97	МН	
105	P-190	6.67	1.56	0.95	7.49	0.22	12	Cir	41	32.57	33.79	2.99	33.96	34.32 j	n/a	34.32	97	МН	
106	P-167	6.68	1.56	0.95	7.49	0.22	12	Cir	42	32.51	33.76	3.00	33.16	34.29 j	n/a	34.29	93	МН	
107	P-178	5.80	1.56	0.95	7.49	0.22	12	Cir	27	32.30	32.90	2.26	33.51	33.47	0.18	33.65	77	МН	
108	P-184	5.79	1.56	0.95	7.49	0.22	12	Cir	28	32.30	32.94	2.25	33.51	33.47	0.21	33.68	77	МН	
109	P-155	3.55	1.27	0.74	7.49	0.23	12	Cir	21	32.00	32.21	0.99	32.41	32.69	0.19	32.69	1	Dp-Grate	
110	P-84	3.84	2.24	0.00	0.00	0.00	12	Cir	44	30.87	31.31	0.99	31.42	31.95	n/a	31.95	3	МН	
111	P-85	3.88	0.57	0.95	7.49	0.08	12	Cir	44	31.31	31.75	1.01	31.95	32.06 j	n/a	32.06	110	МН	
112	P-83	3.85	1.78	0.95	7.49	0.25	12	Cir	169	31.31	33.00	1.00	31.95	33.57 j	n/a	33.57	110	МН	
113	P-169	6.68	1.56	0.95	7.49	0.22	12	Cir	42	31.16	32.41	3.00	31.95	32.94 j	n/a	32.94	92	MH	
114	P-70	23.07	3.09	0.81	7.49	0.48	24	Cir	198	27.56	29.62	1.04	29.11	30.23 j	n/a	30.23	3	Dp-Grate	
115	P-68	10.03	0.75	0.81	7.49	0.21	18	Cir	47	29.70	30.13	0.91	30.23	30.45 j	n/a	30.45	114	Dp-Grate	
Projec	t File: 190826_	_PipeSizir	ig.stm	<u>I</u>	1	1	<u> </u>	L			L	1	Num	ber of lines	: 124	L	Da	ute: 8/26/201	9
NOTES: Intensity = 182.59 / (Inlet time + 19.10) ^ 0.99 Return period = 25 Yrs. ; ** Critical depth											<b>I</b>								

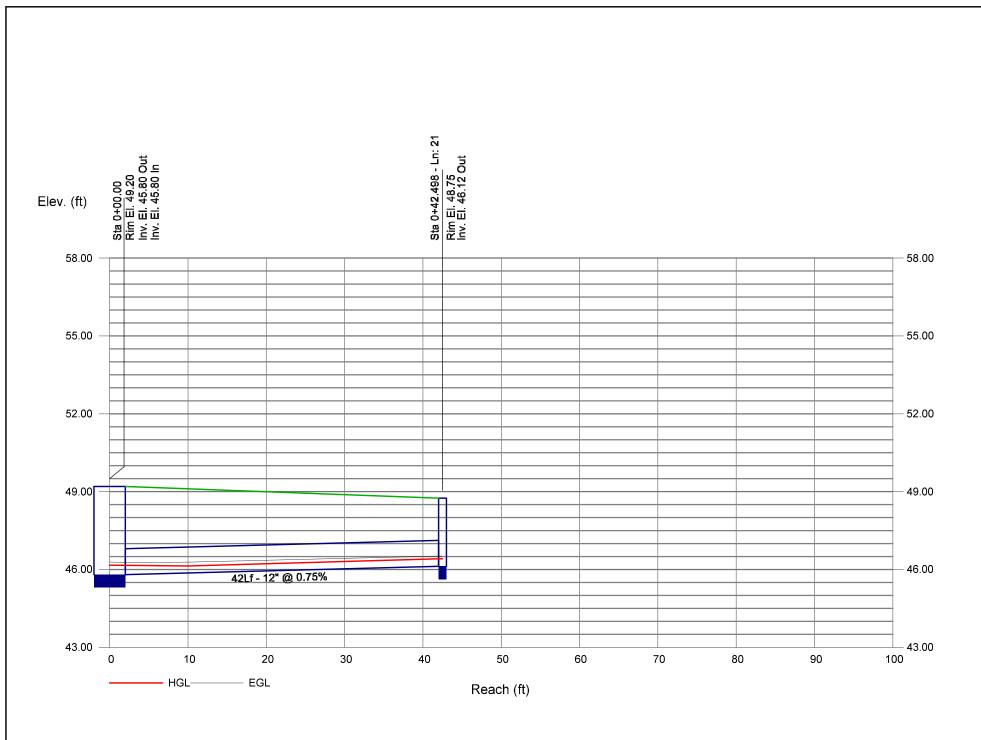
## **MyReport**

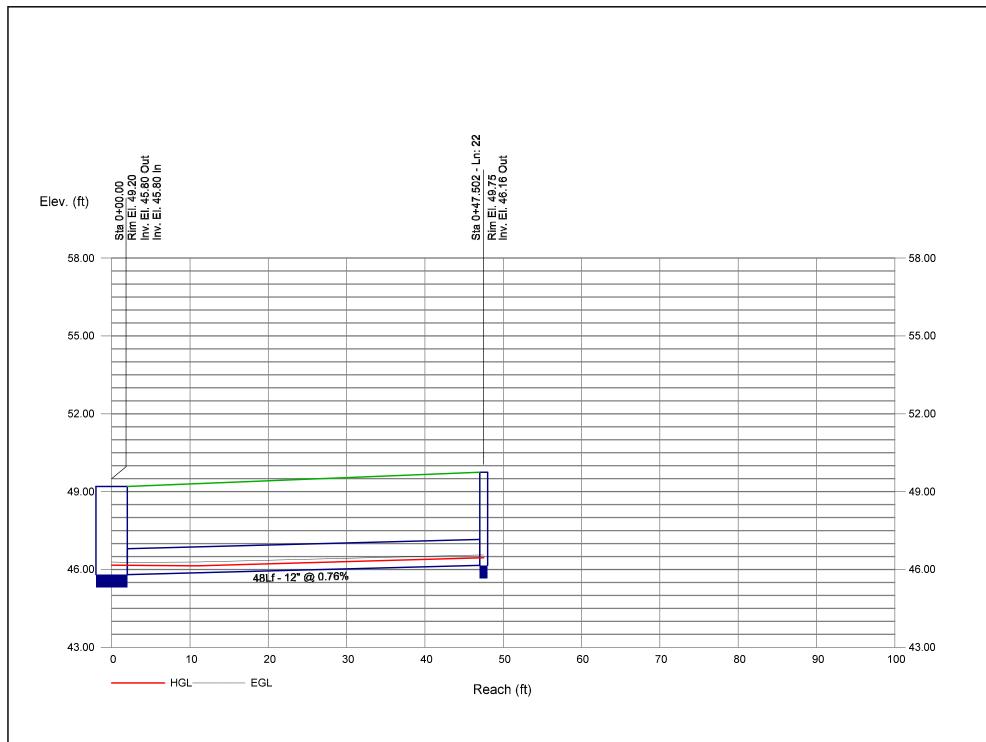
Line No.	Line ID	Capac Full	Flow Rate	Runoff Coeff	i Inlet	Drng Area	Line Size	Line Type	Line Length	Invert Dn	Invert Up	Line Slope	HGL Dn	HGL Up	Minor Loss	HGL Jnct	DnStm Ln No	Junct Type
		(cfs)	(cfs)	(C)	(in/hr)	(ac)	(in)		(ft)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)		
116	P-34	10.19	0.13	0.90	7.49	0.02	18	Cir	103	30.21	31.18	0.94	30.45	31.31 j	n/a	31.31	115	Dp-Grate
117	P-80	9.14	1.63	0.87	7.49	0.25	15	Cir	103	30.07	32.14	2.00	30.43	32.65	0.19	32.65	114	Dp-Grate
118	P-164	5.04	1.53	0.00	0.00	0.00	12	Cir	52	28.90	29.95	2.00	30.38	30.48 j	n/a	30.67	4	None
119	P-163	5.03	1.56	0.95	7.49	0.22	12	Cir	75	29.95	31.45	2.00	30.67	31.98 j	n/a	31.98	118	MH
120	P-172	6.68	1.56	0.95	7.49	0.22	12	Cir	42	29.81	31.06	3.00	30.73	31.59 j	n/a	31.59	91	МН
121	P-42	12.87	2.16	0.90	7.49	0.32	18	Cir	143	29.41	31.56	1.50	29.83	32.11	0.21	32.11	1	Dp-Grate
122	P-76	9.66	2.32	0.81	7.49	0.11	18	Cir	28	27.53	27.77	0.85	28.03	28.35	n/a	28.35	1	Dp-Grate
123	P-75	10.44	1.91	0.81	7.49	0.22	18	Cir	144	27.85	29.27	0.99	28.35	29.79	n/a	29.79	122	Dp-Grate
124	P-74	9.97	0.91	0.81	7.49	0.15	18	Cir	139	29.35	30.60	0.90	29.79	30.95 j	n/a	30.95	123	Dp-Grate
Project	t File: 190826	_PipeSizin	ig.stm										Num	ber of lines	: 124		Da	ate: 8/26/2019

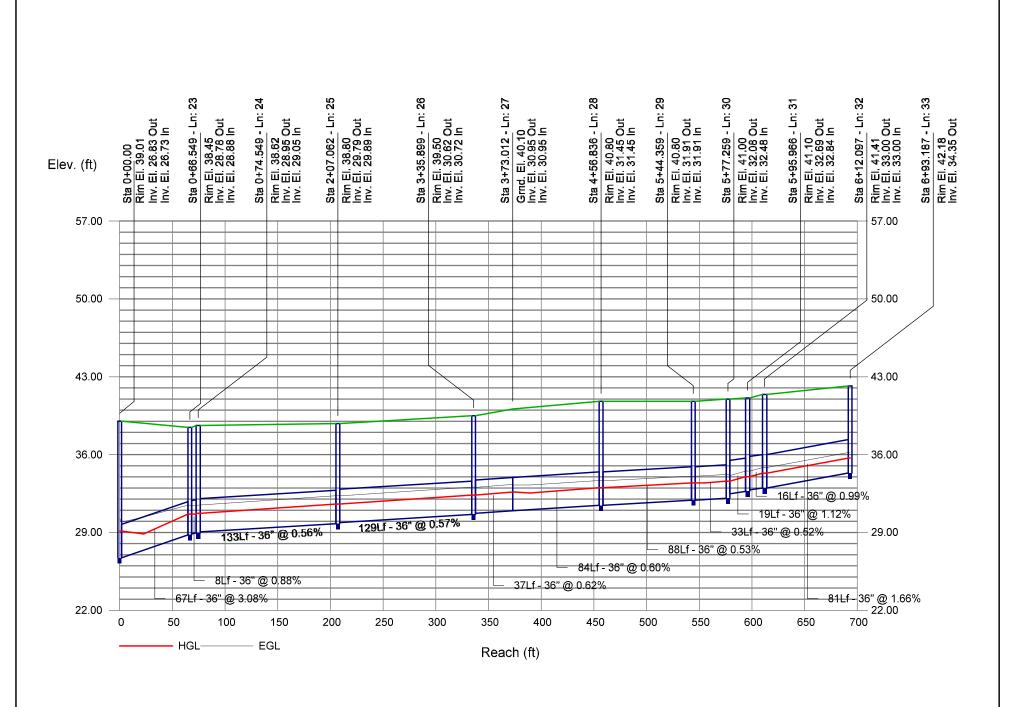


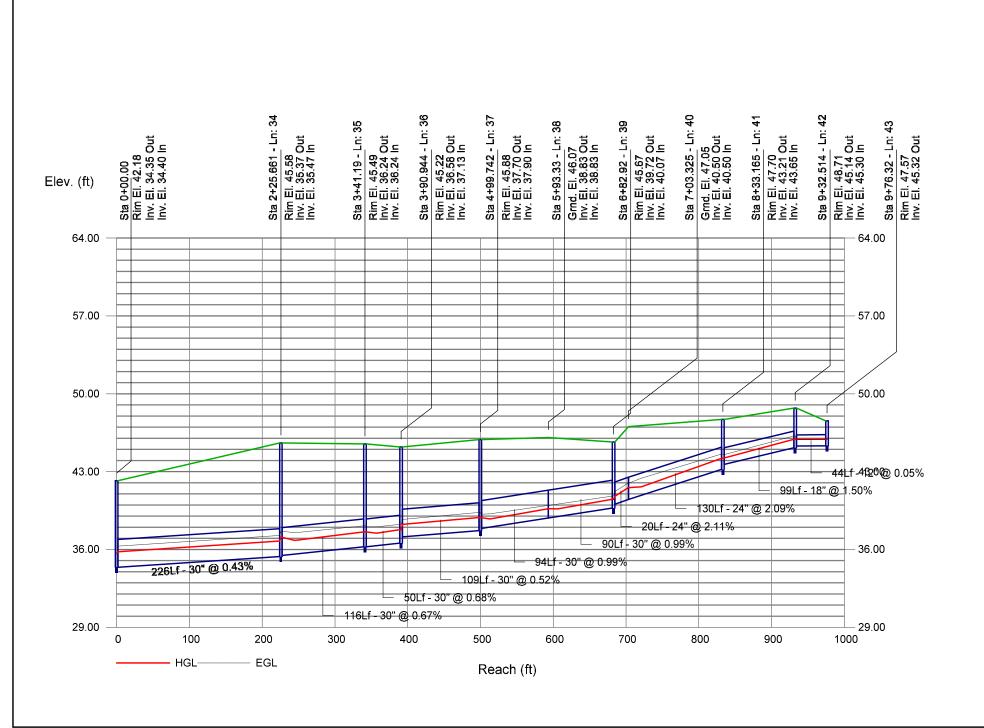


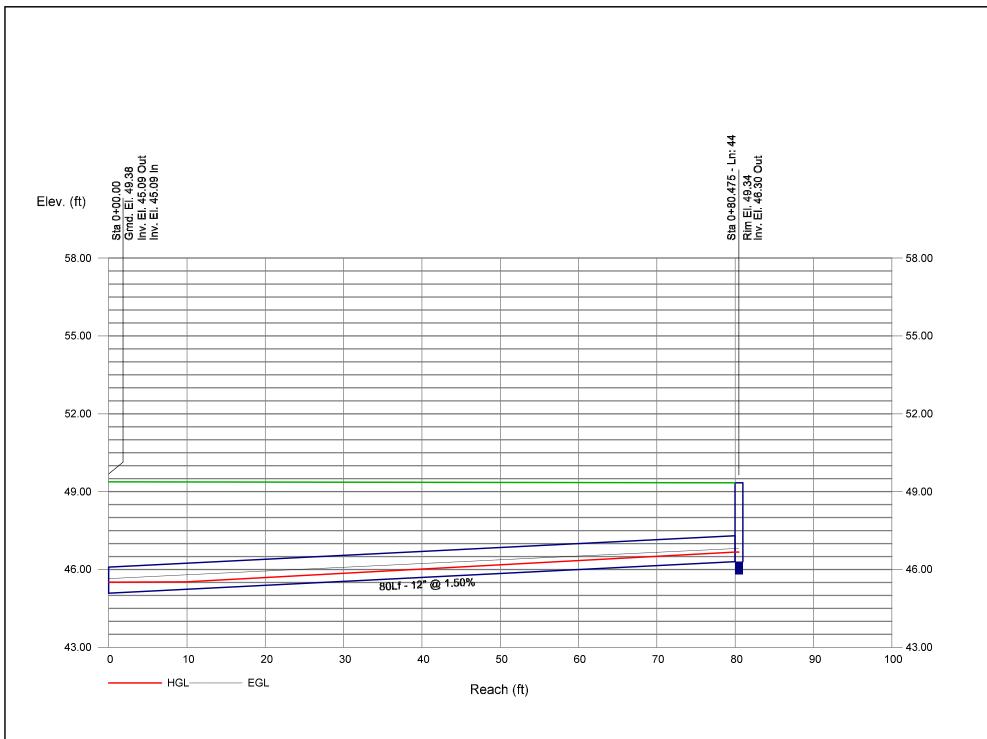


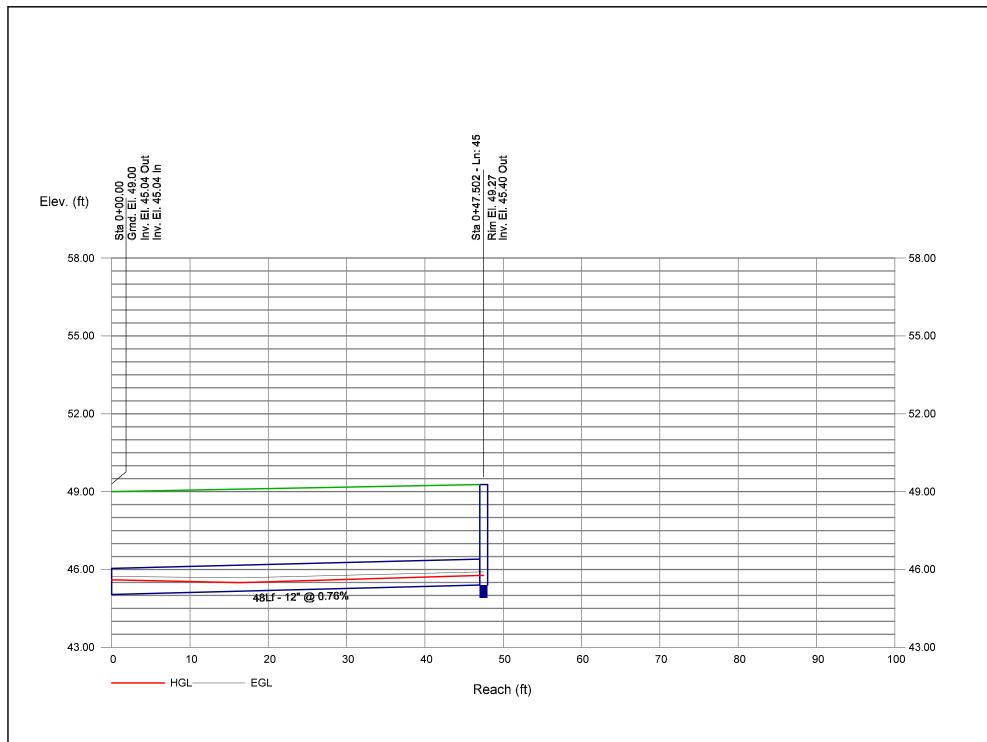


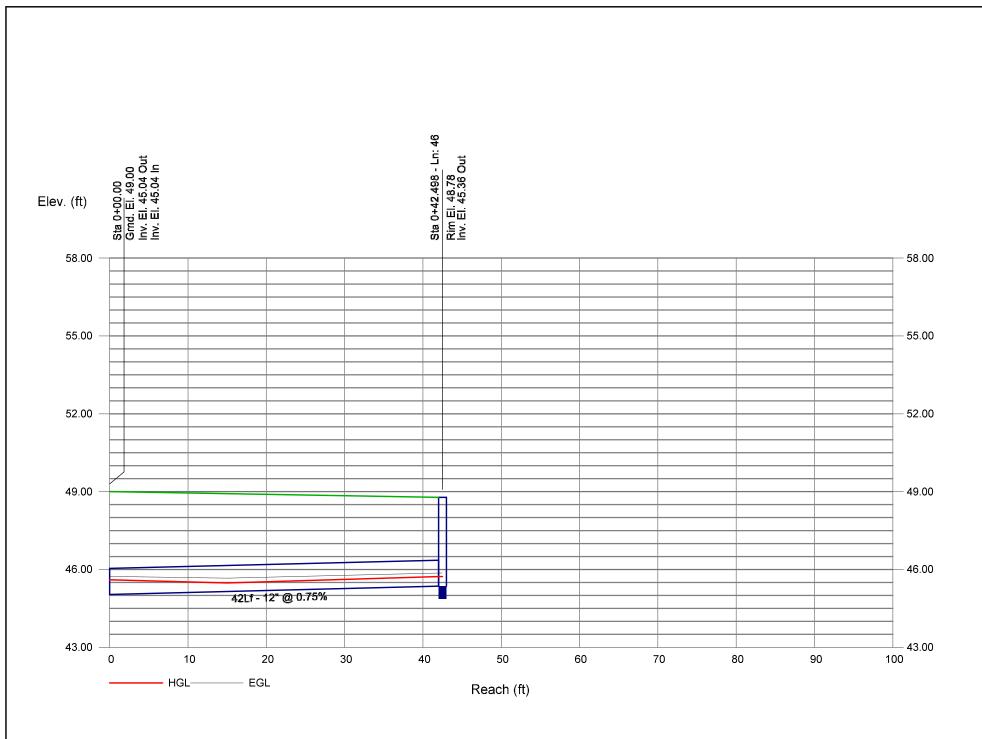


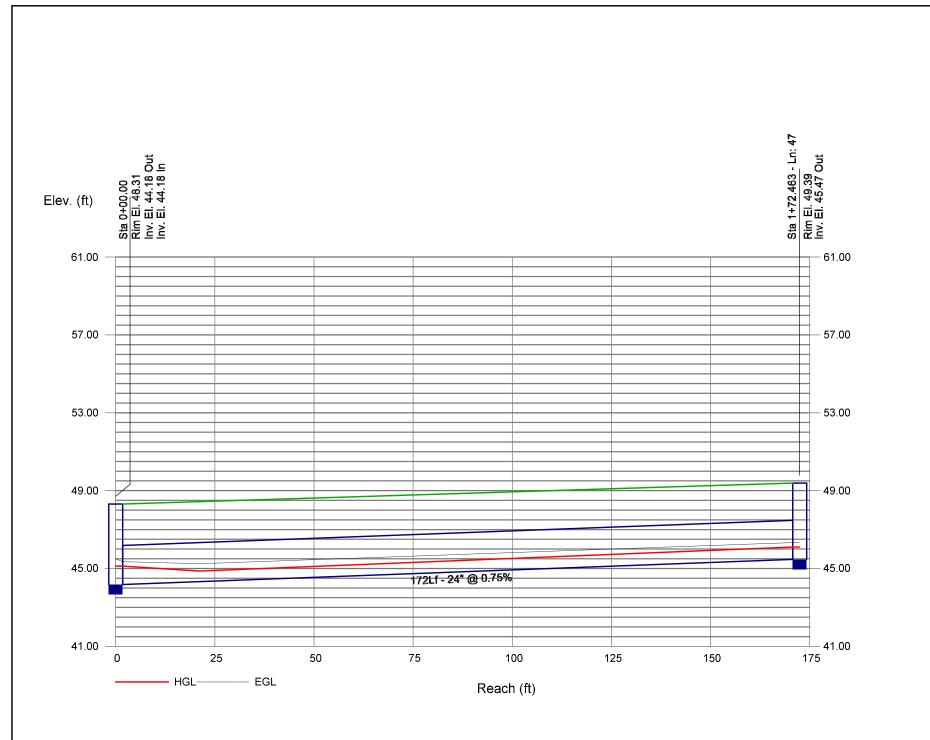


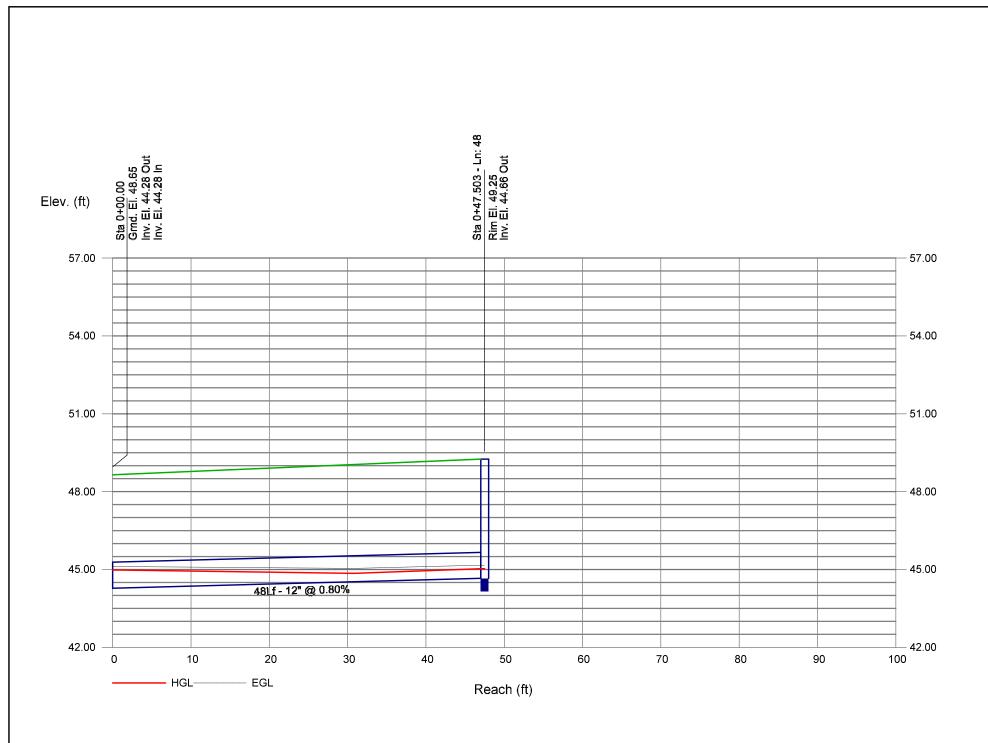


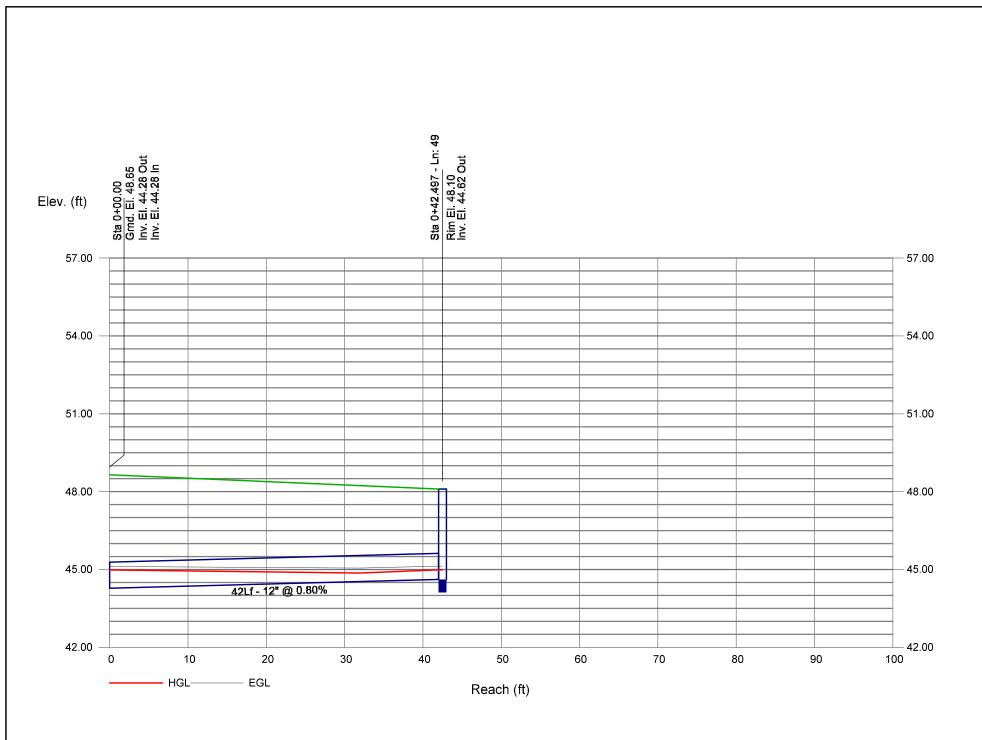


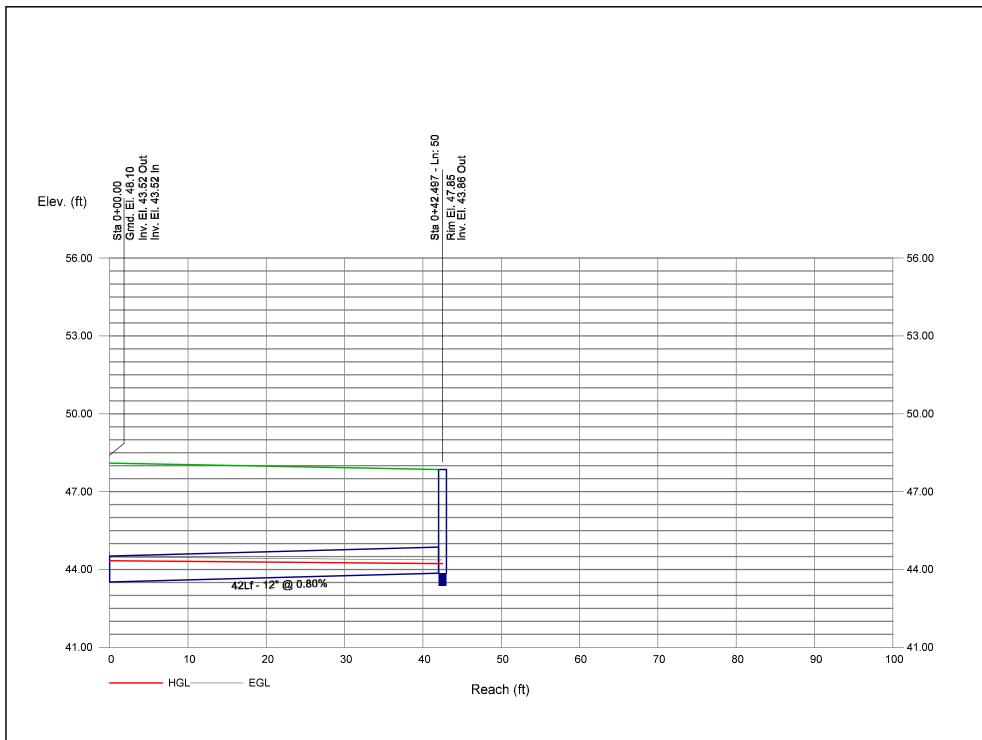


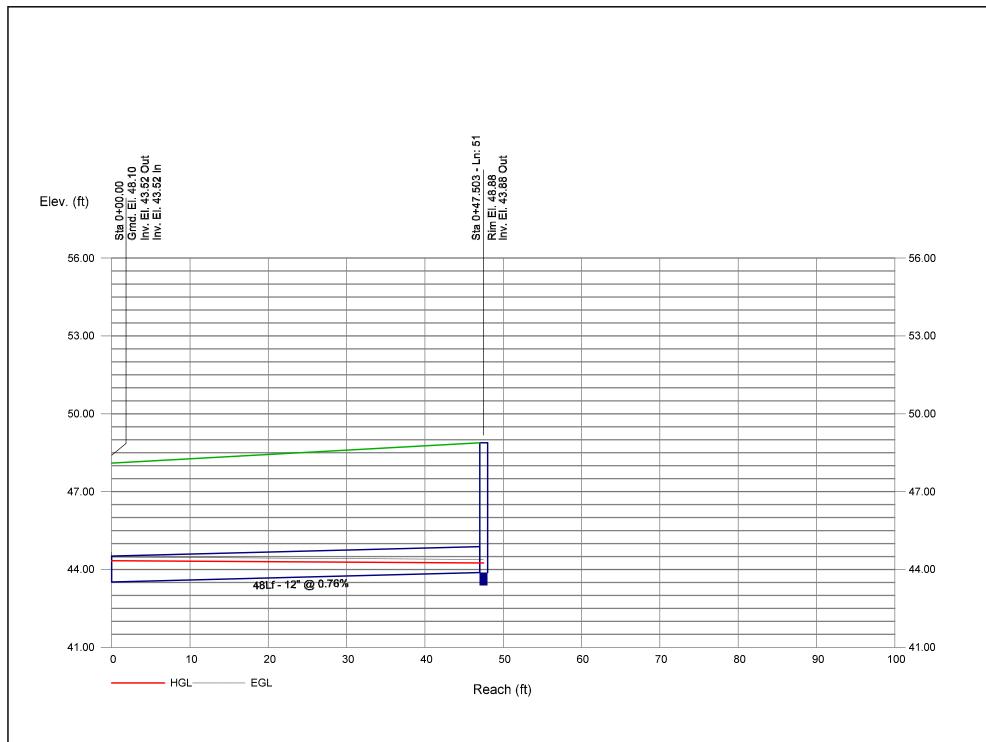


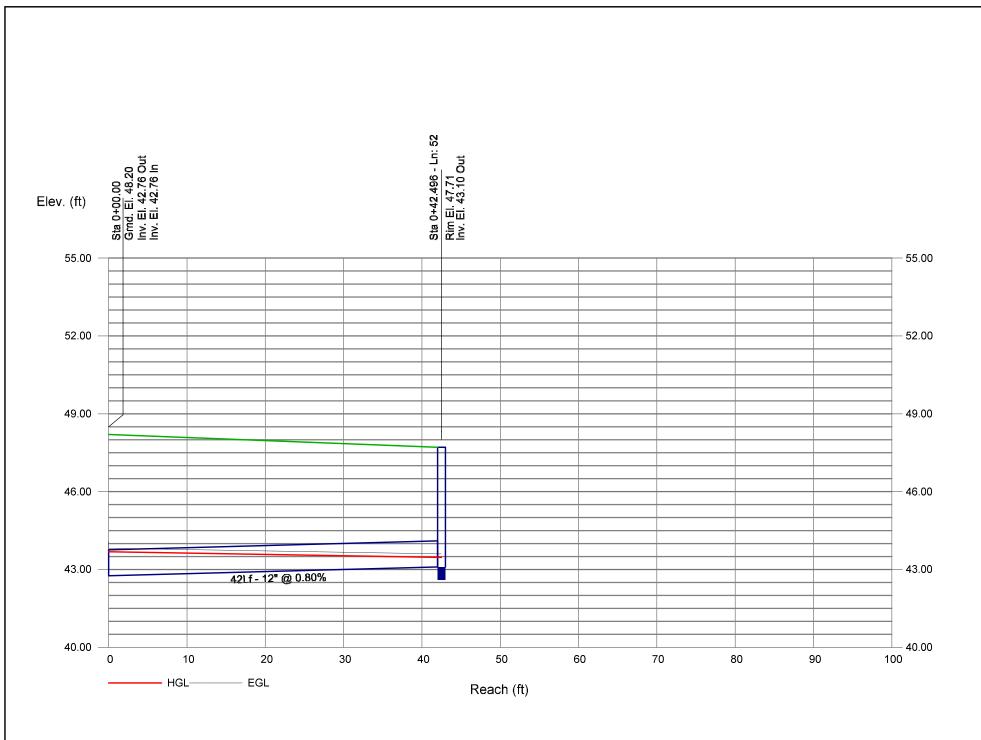


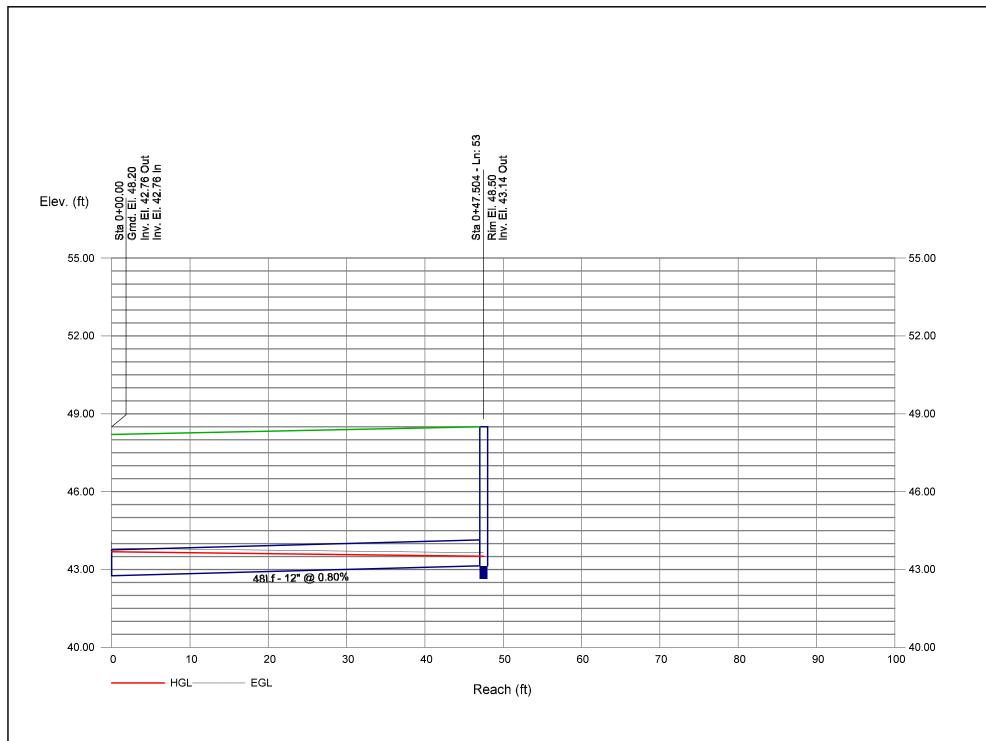


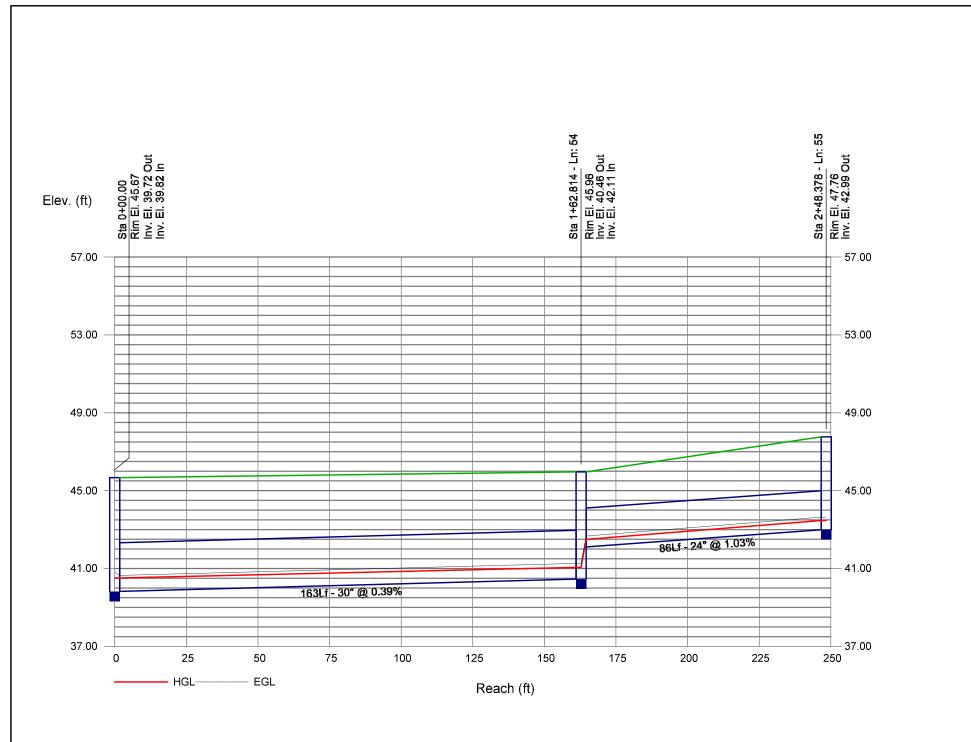


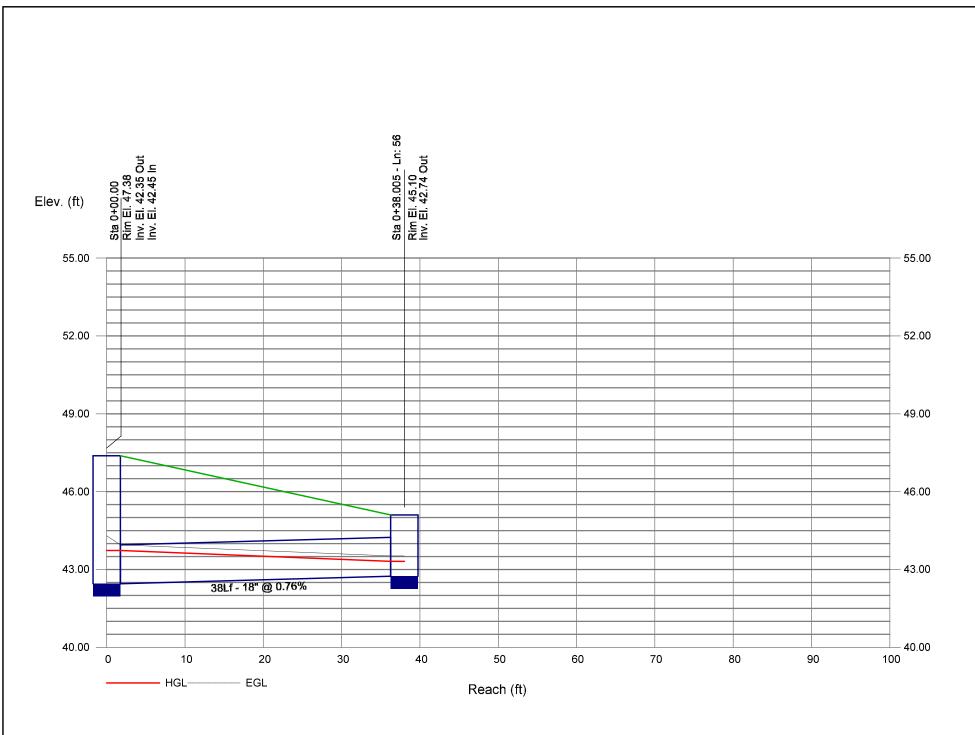


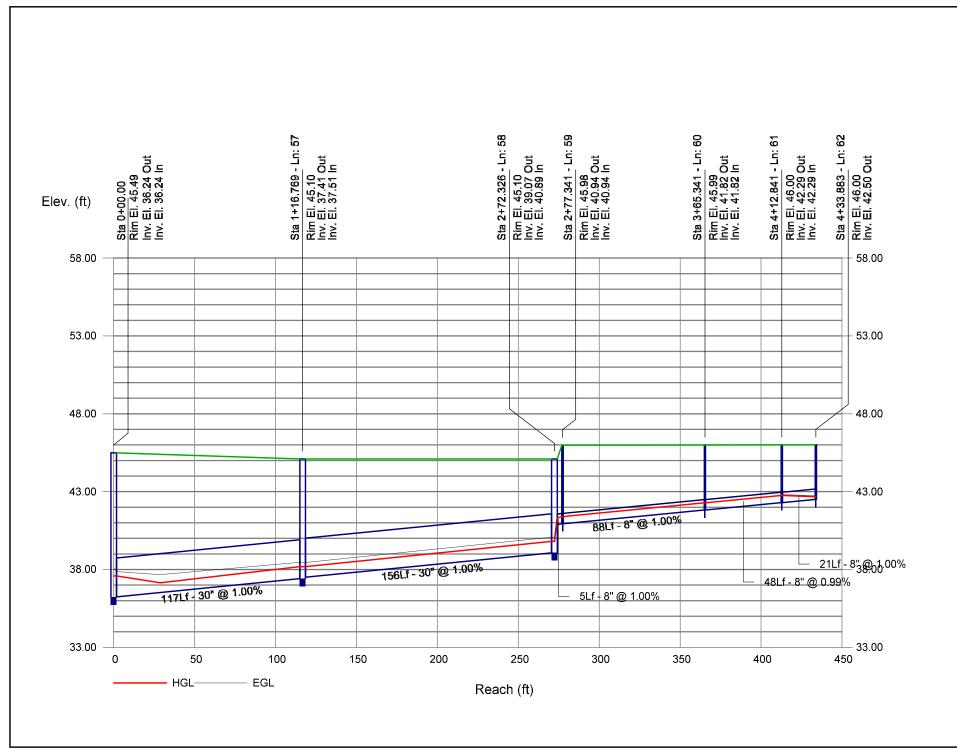


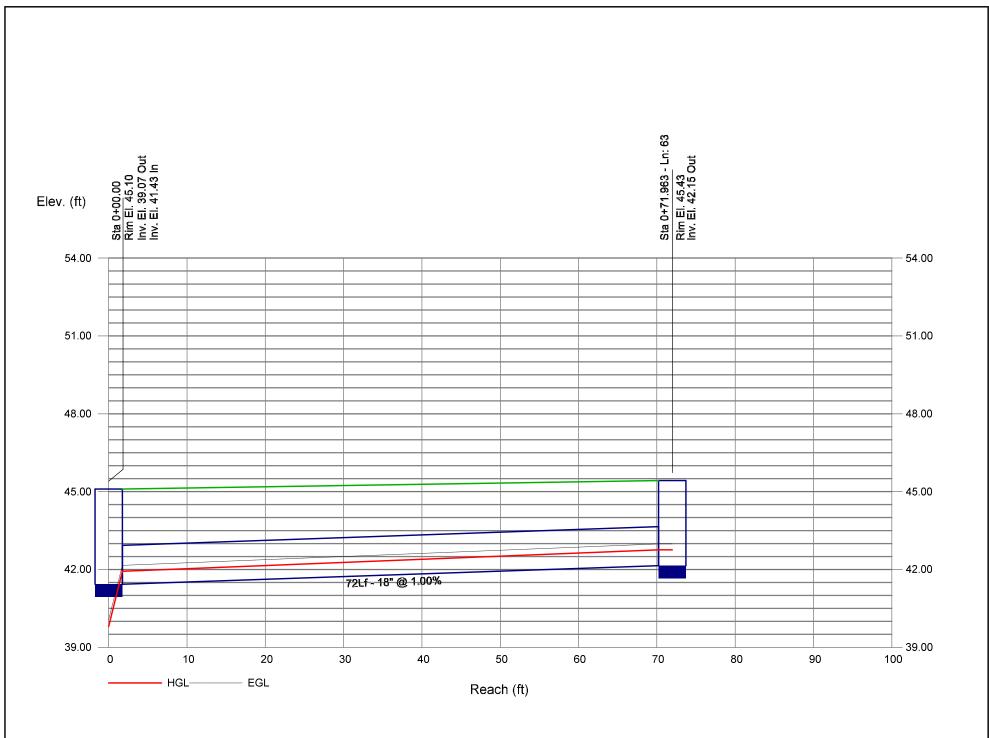


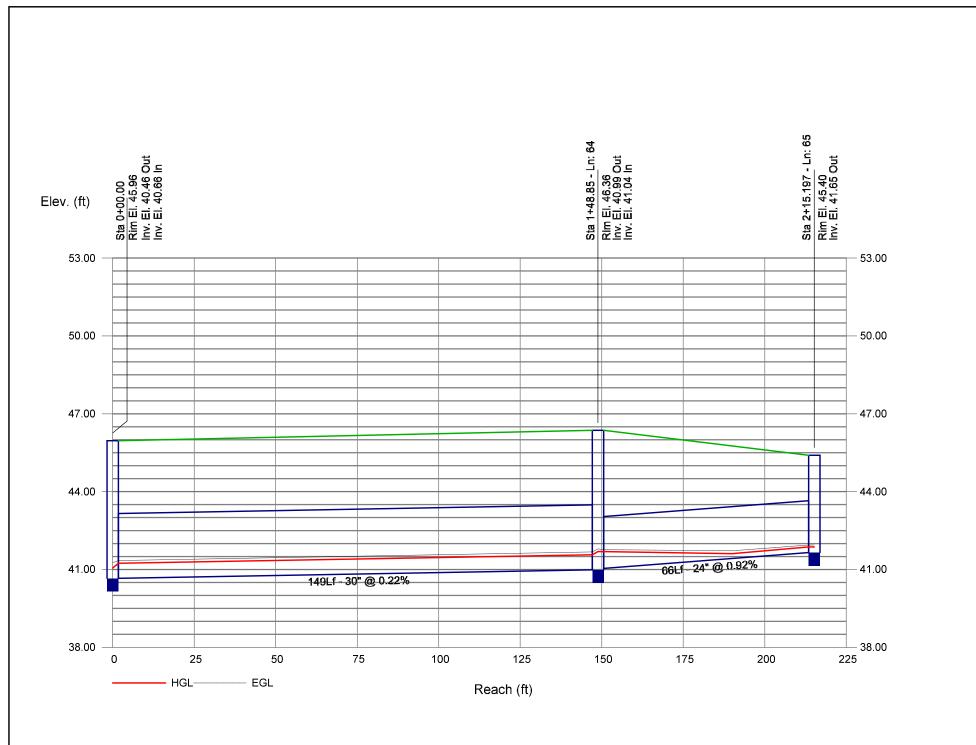


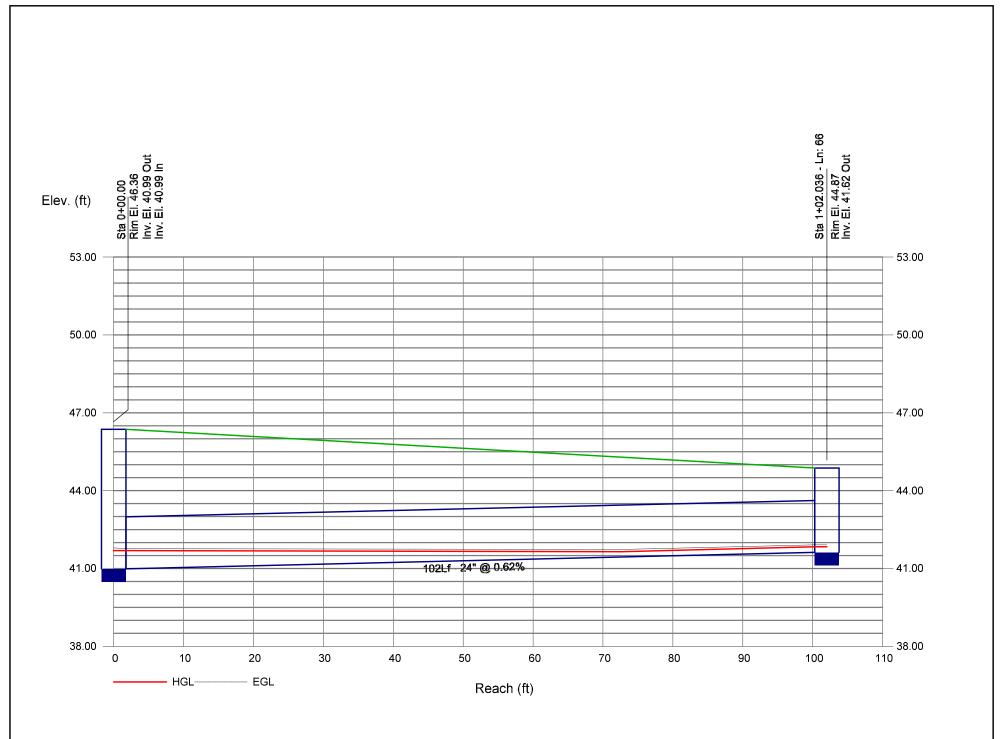


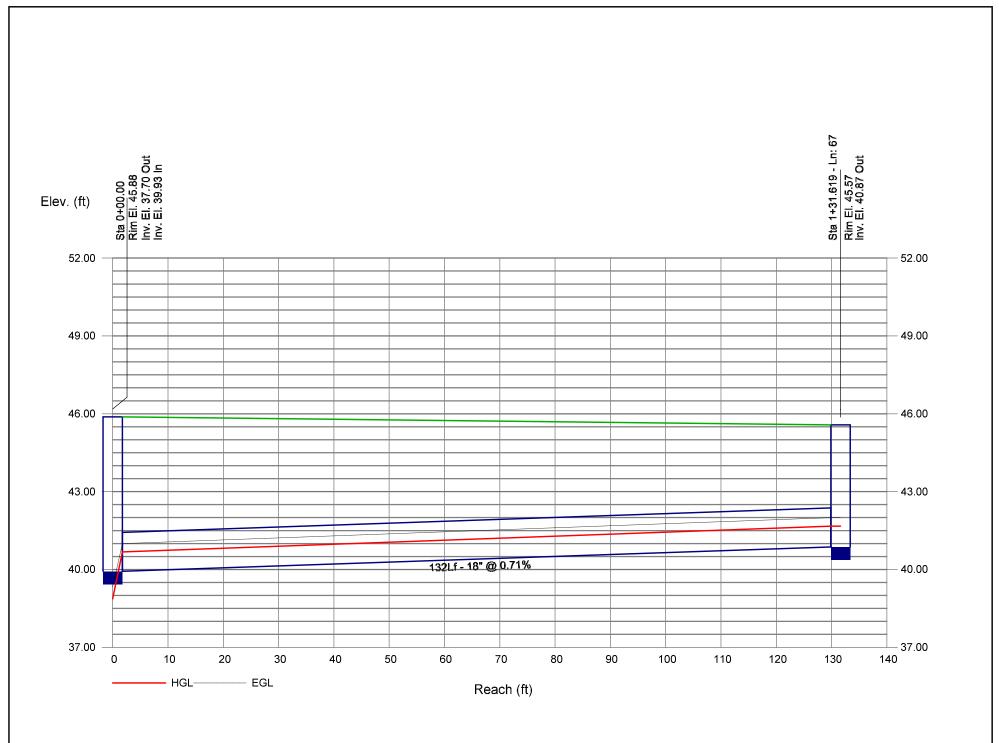


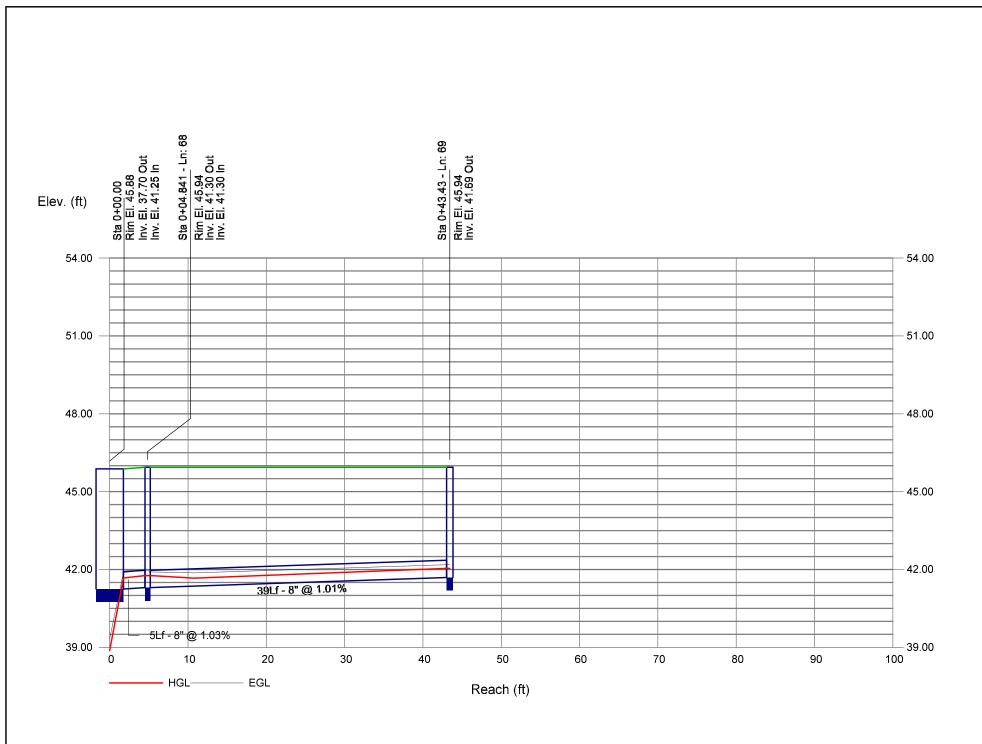


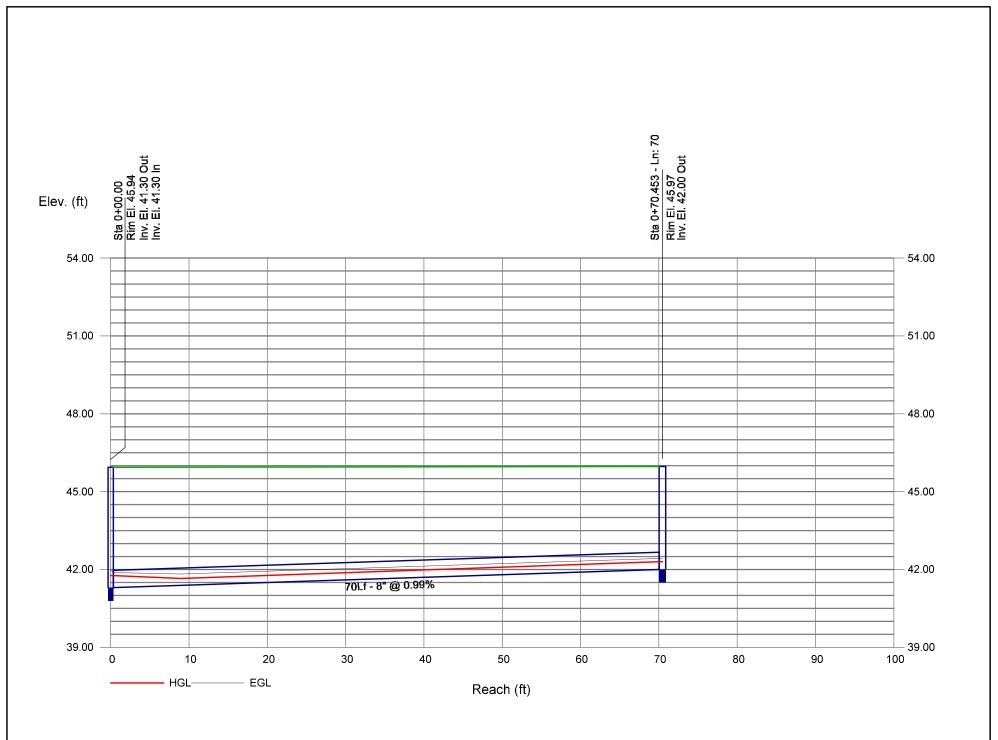


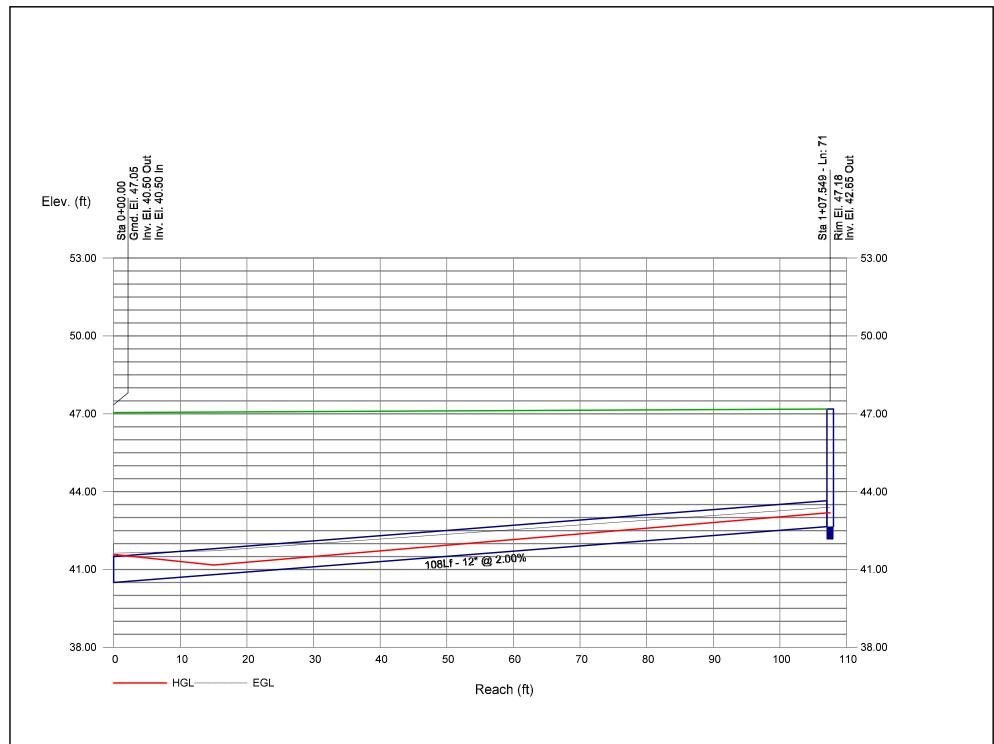


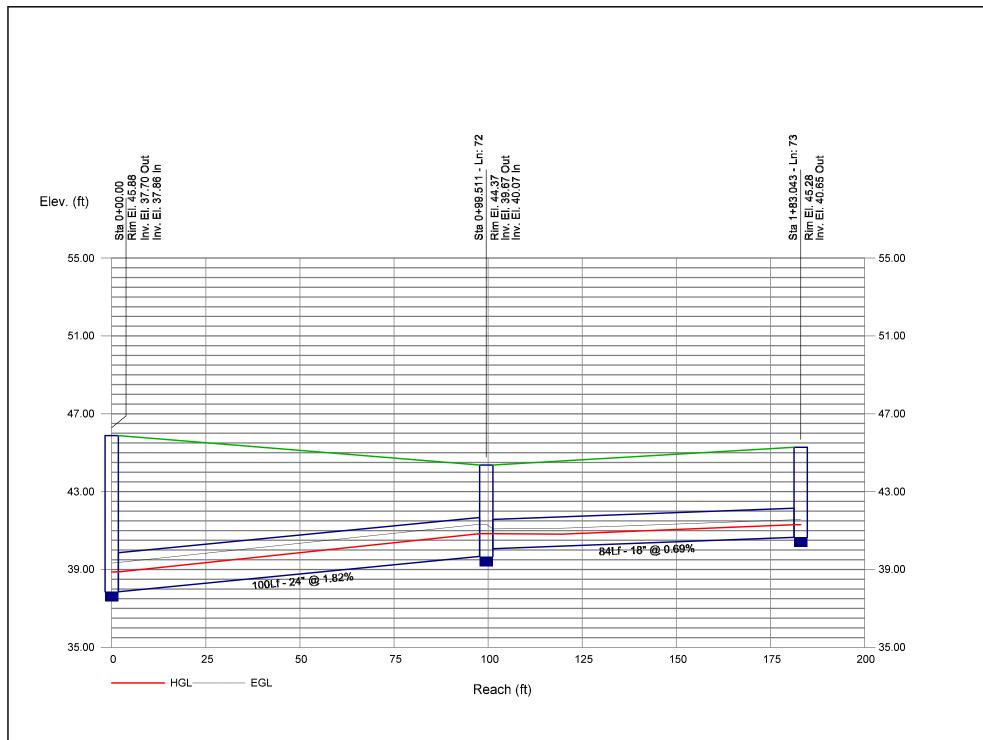


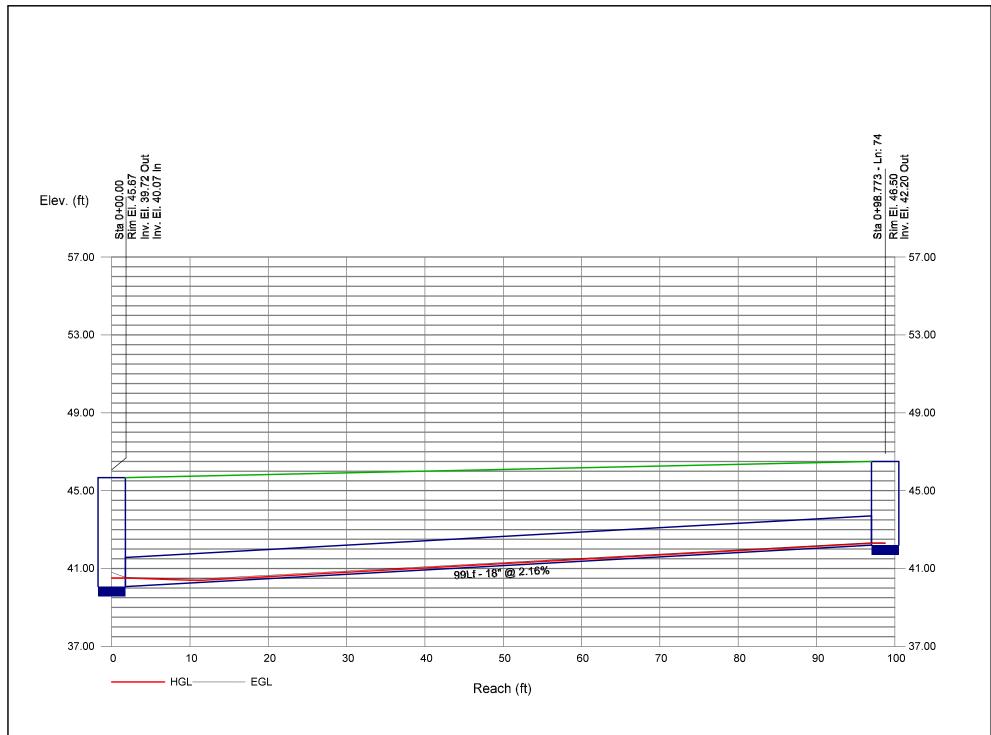


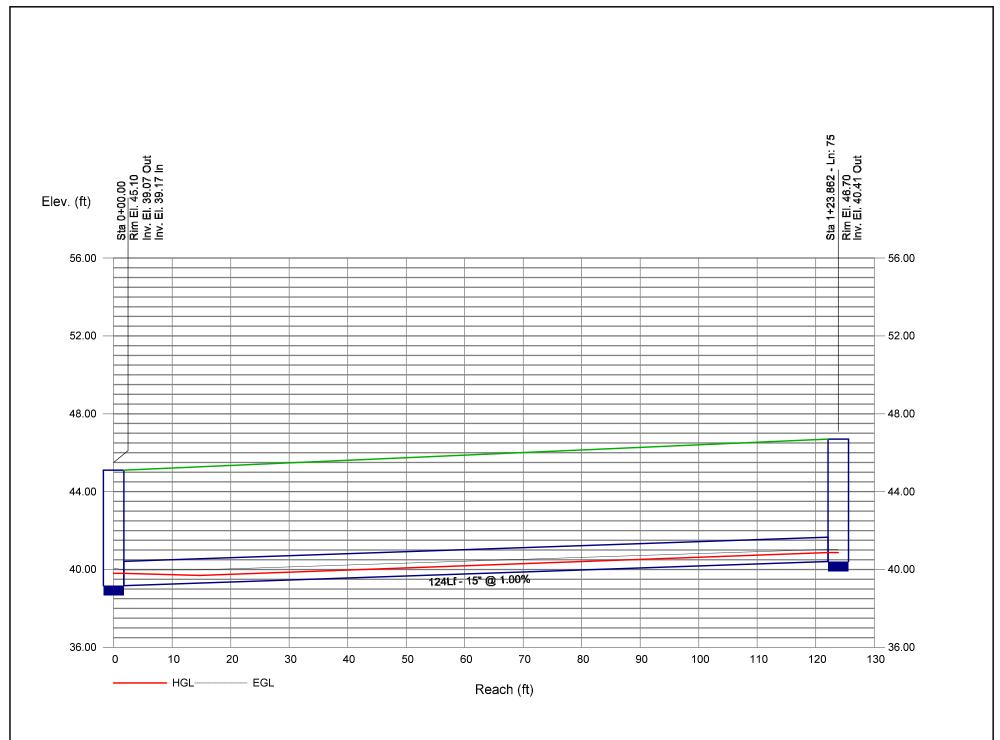


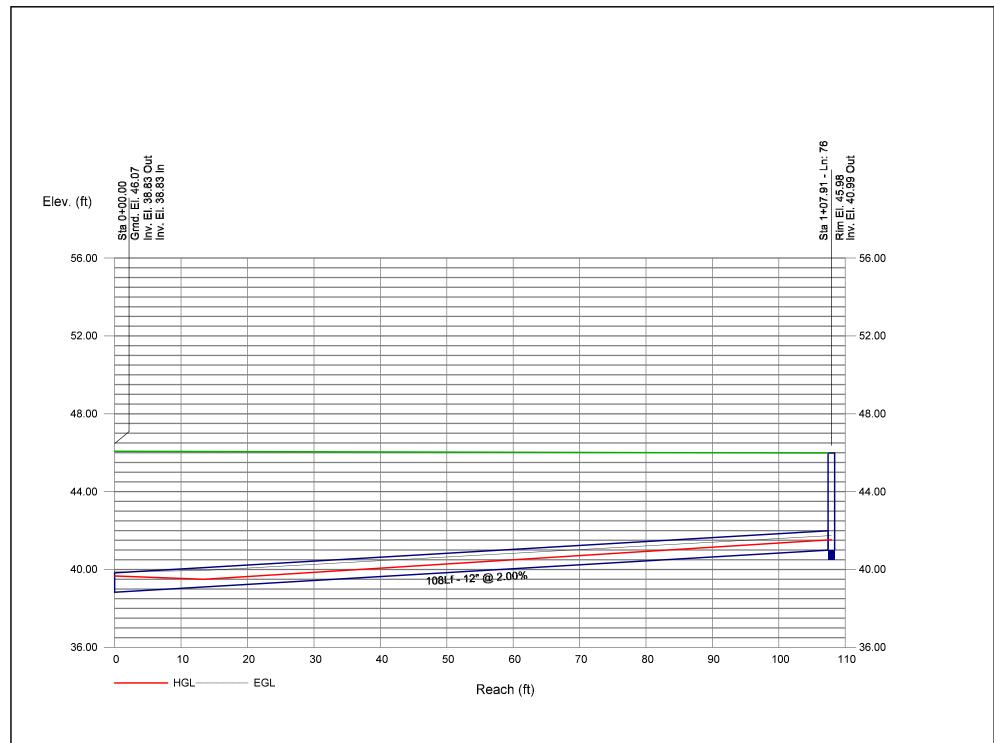


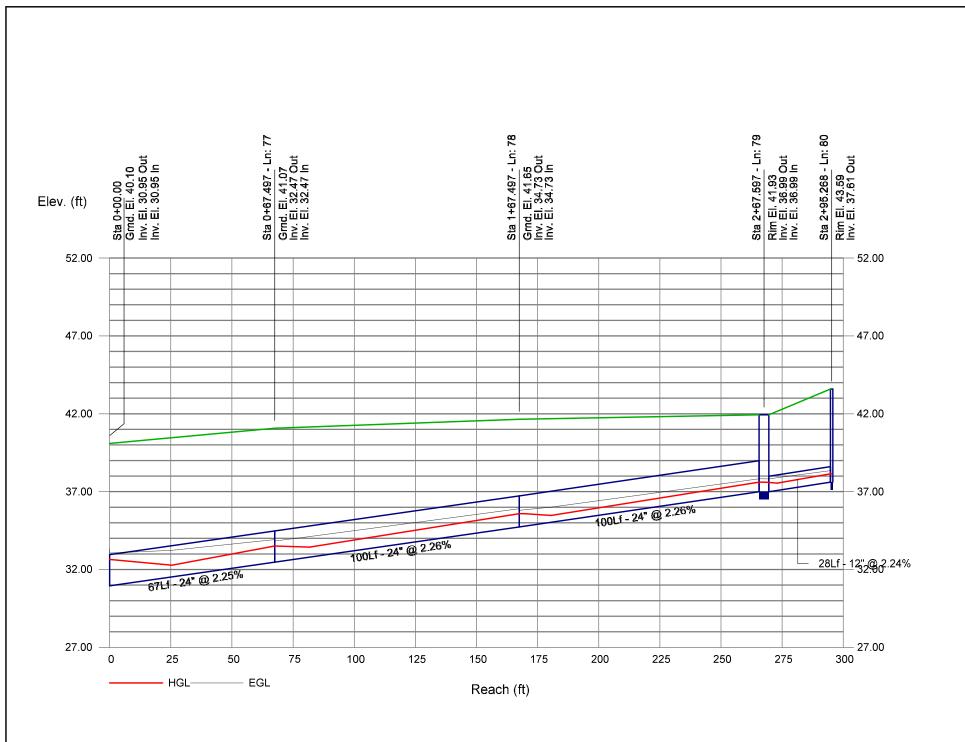


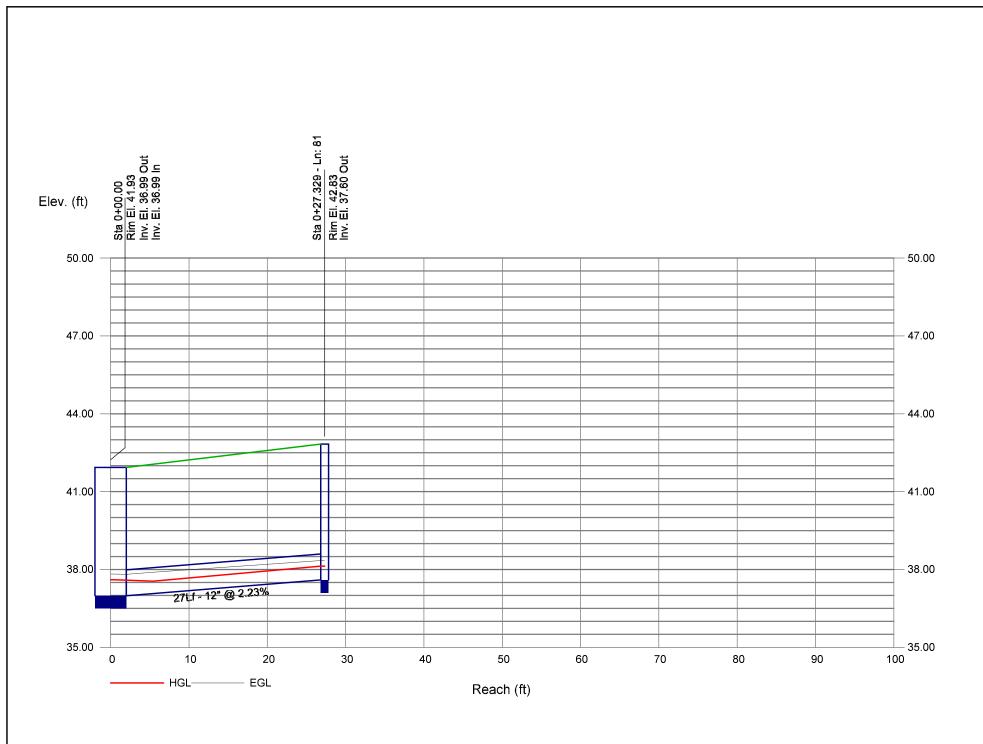


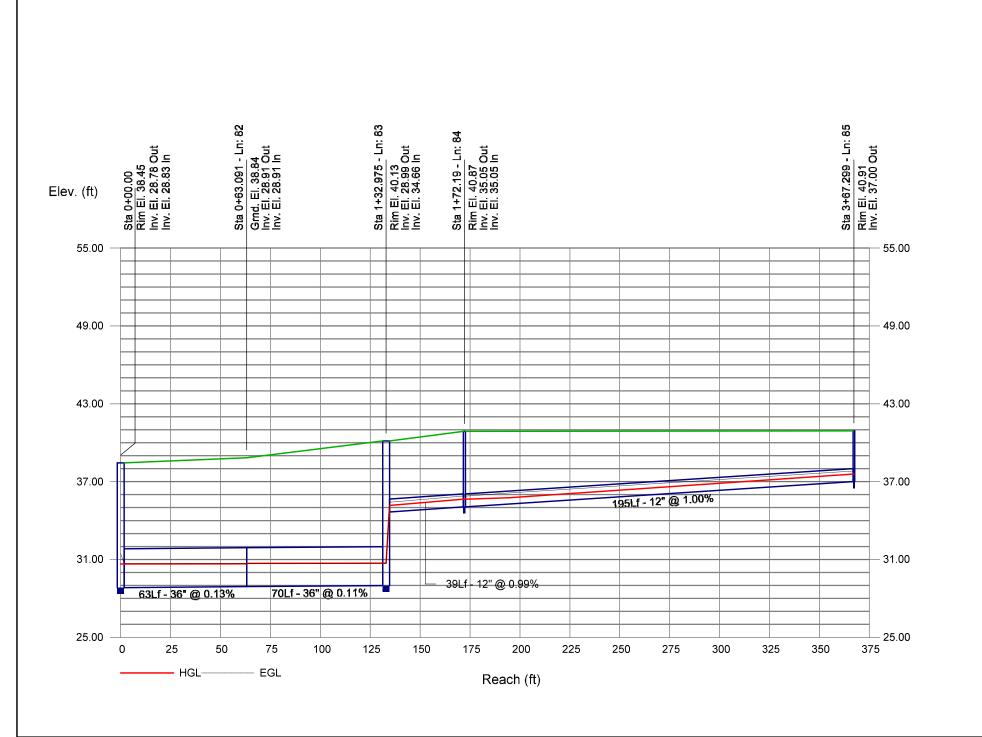


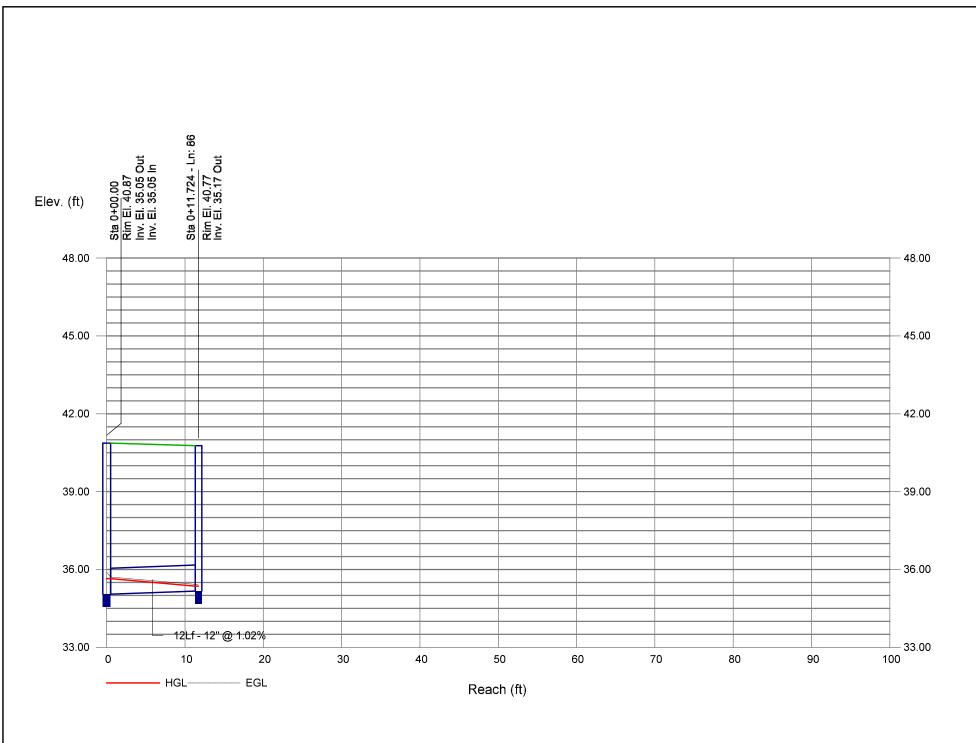


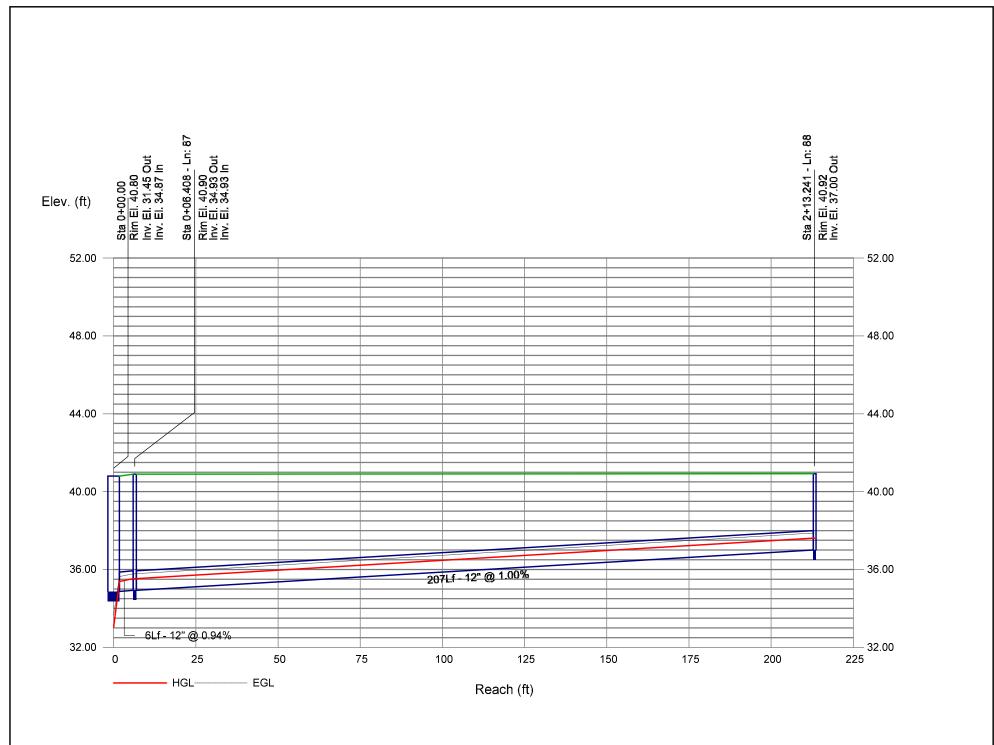


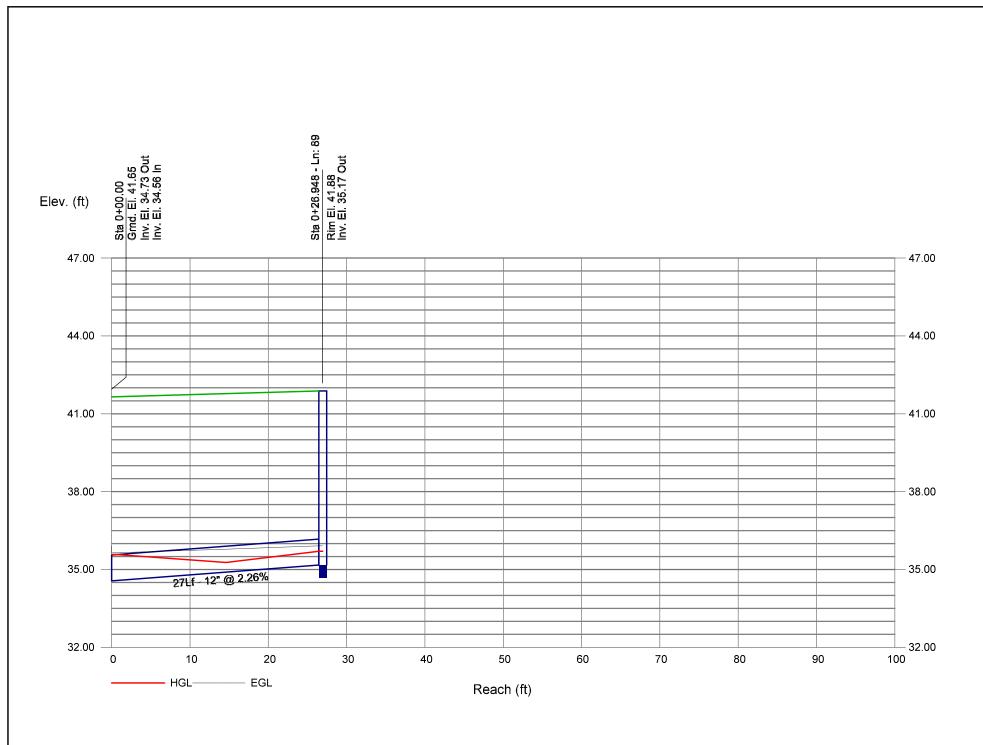


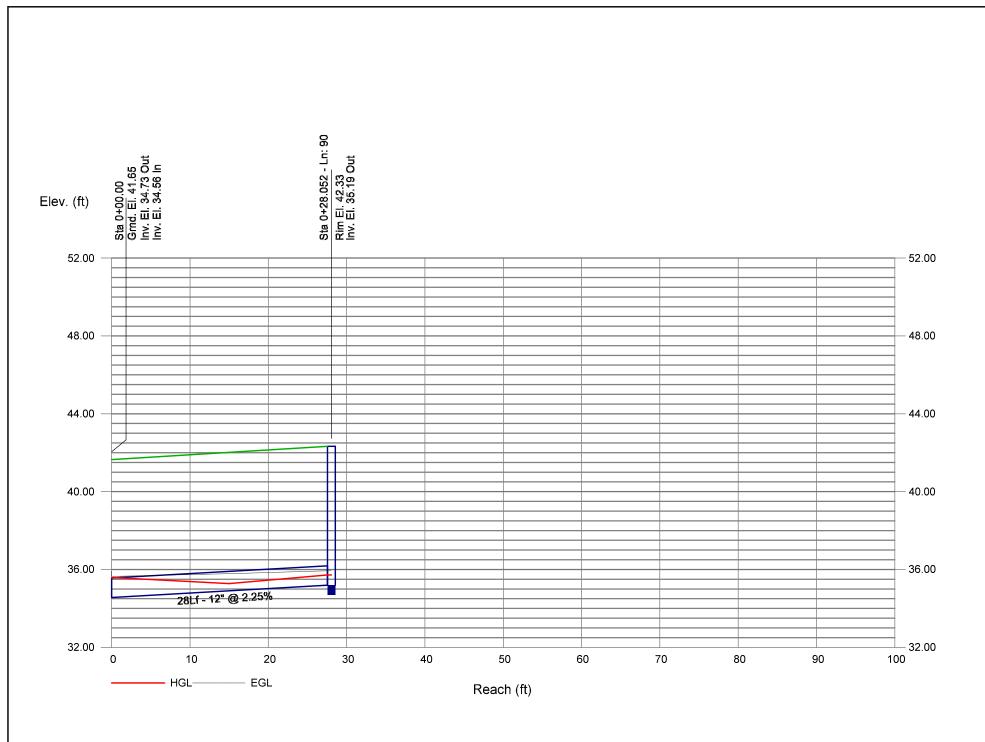


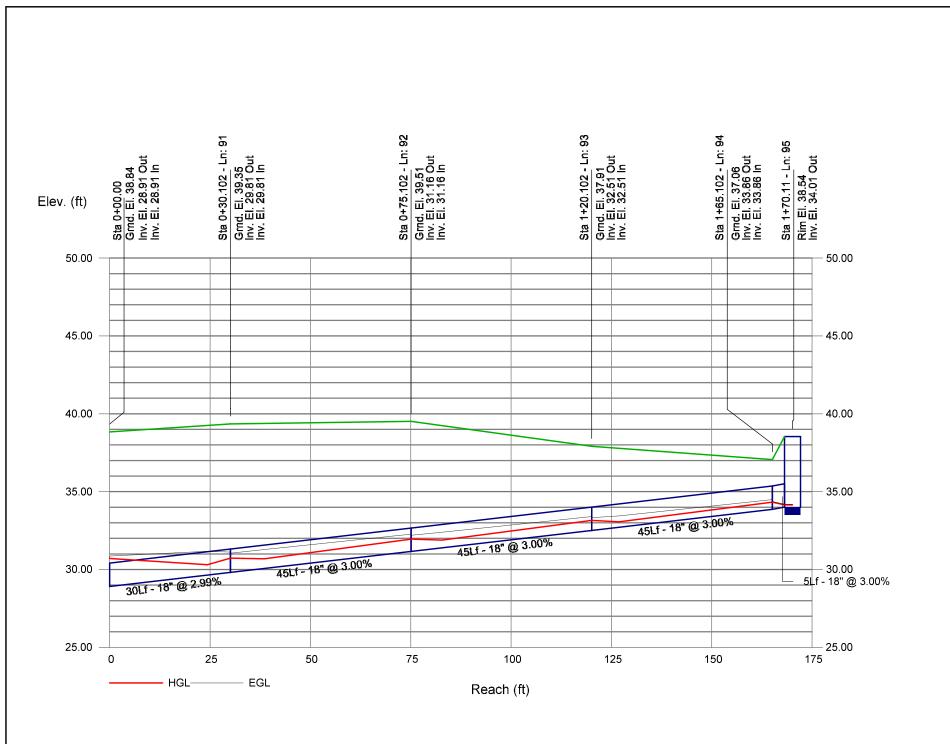


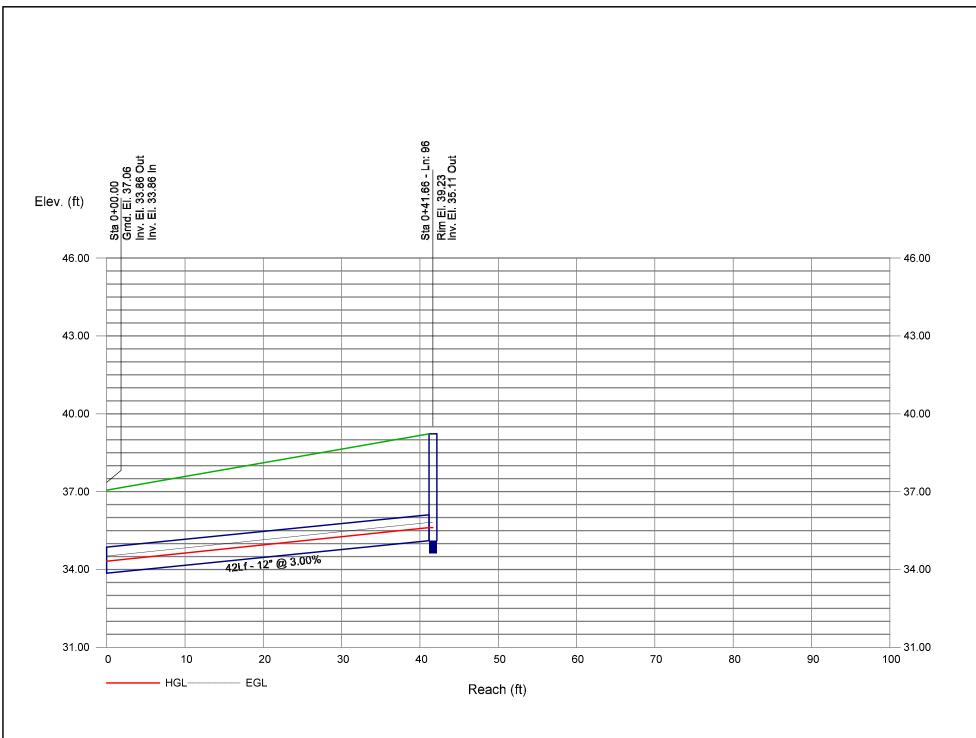


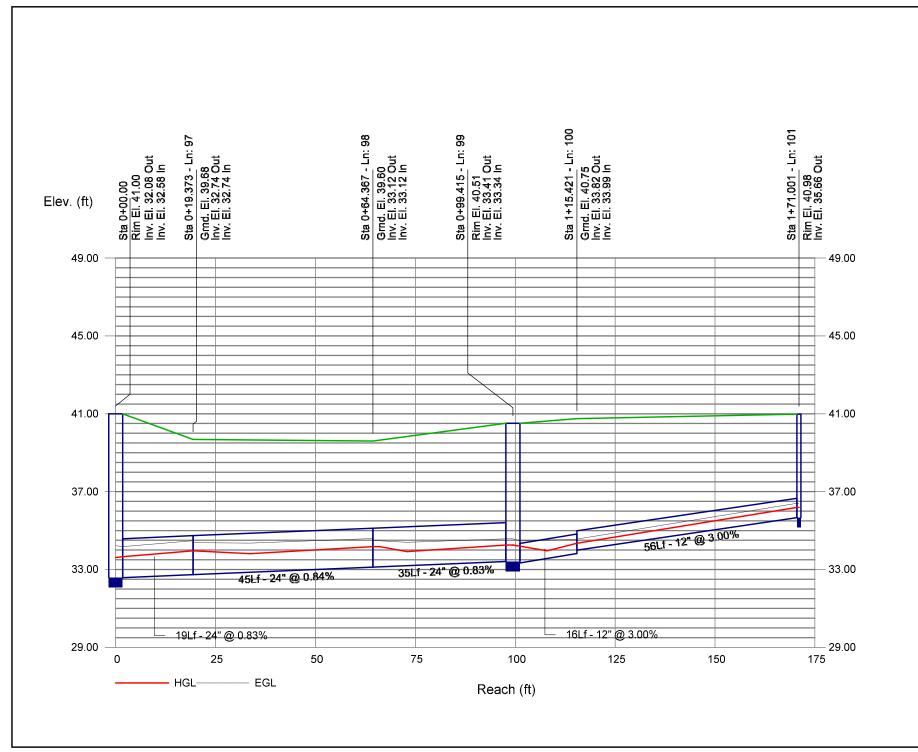


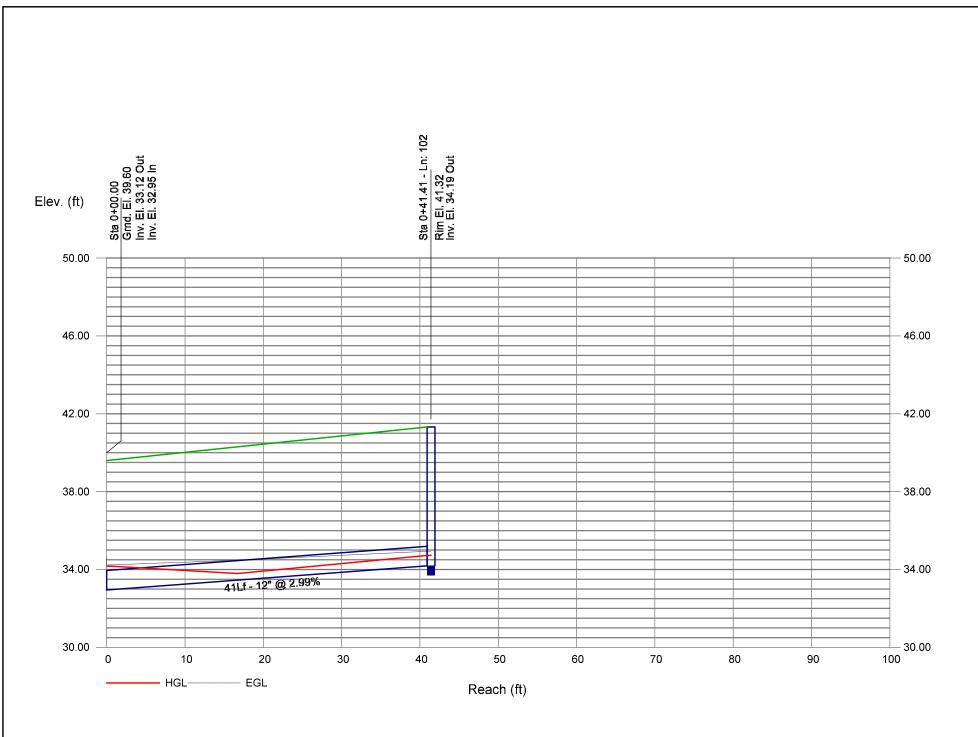


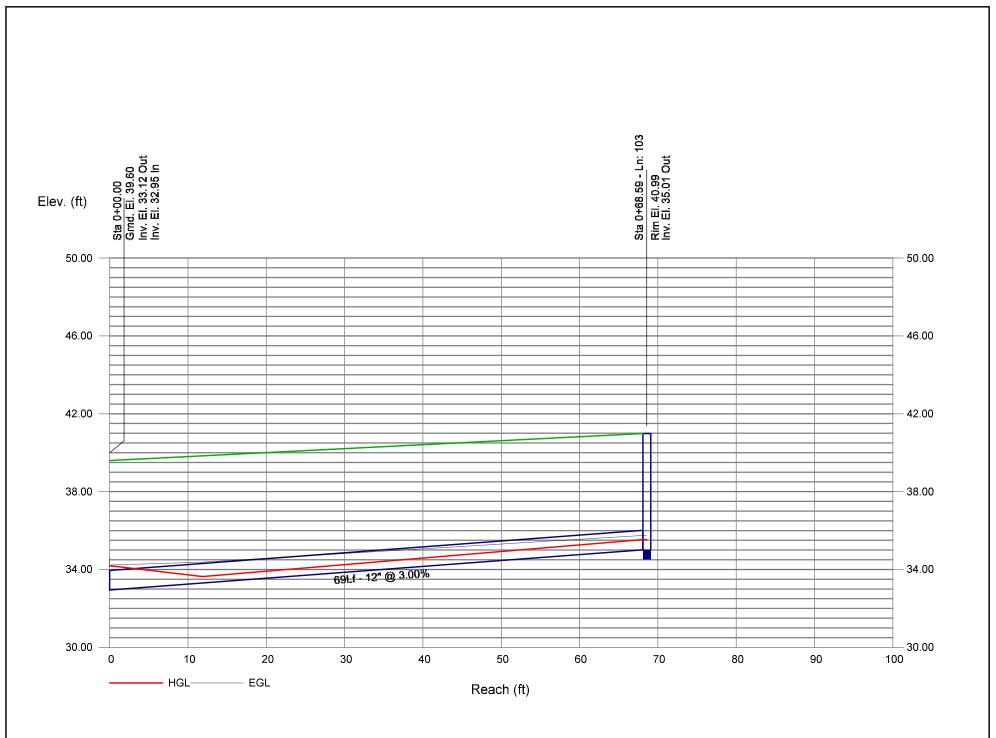


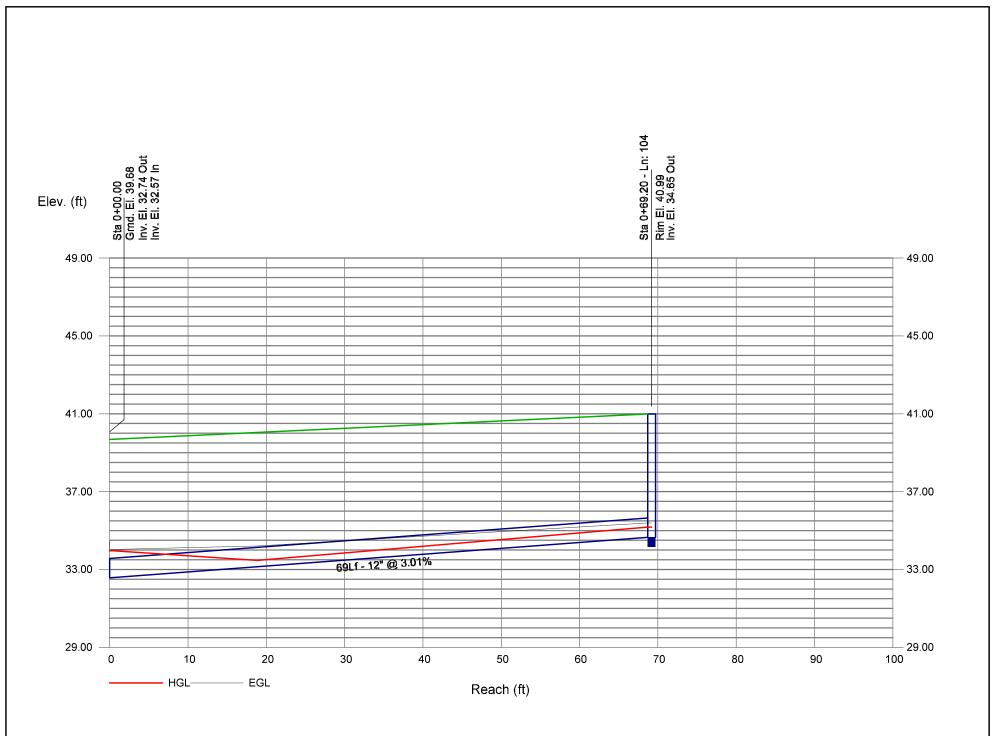


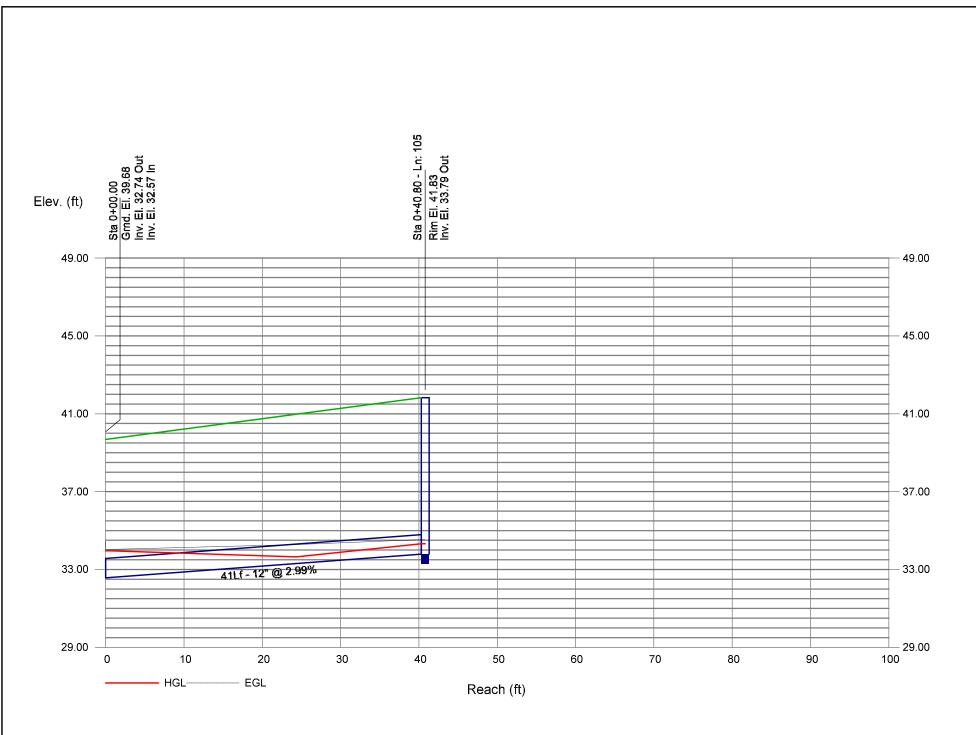


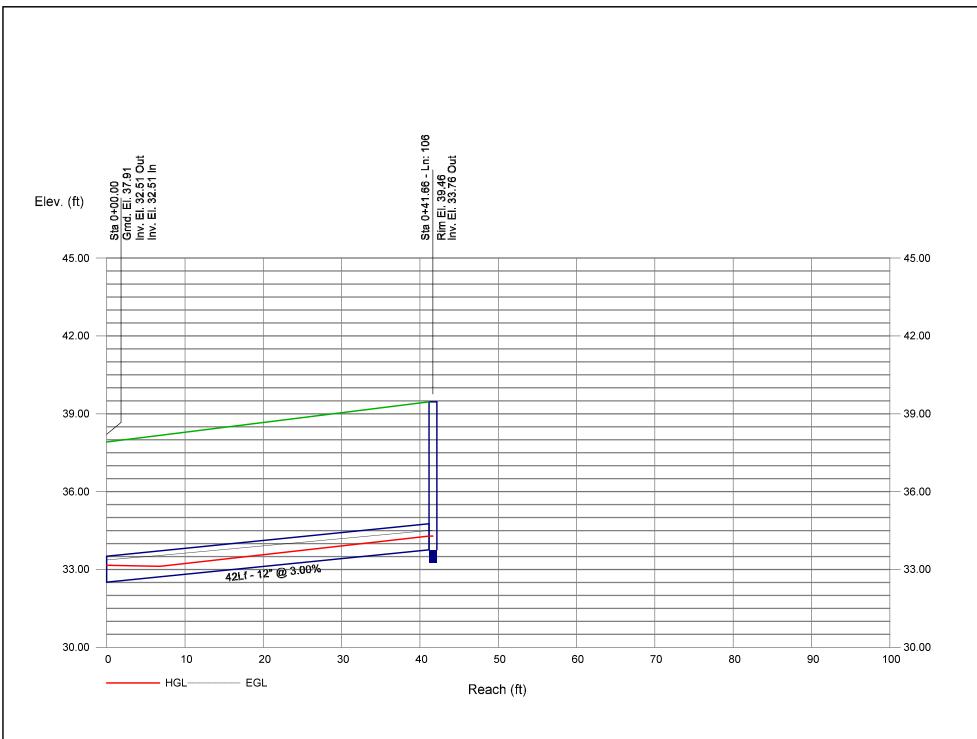


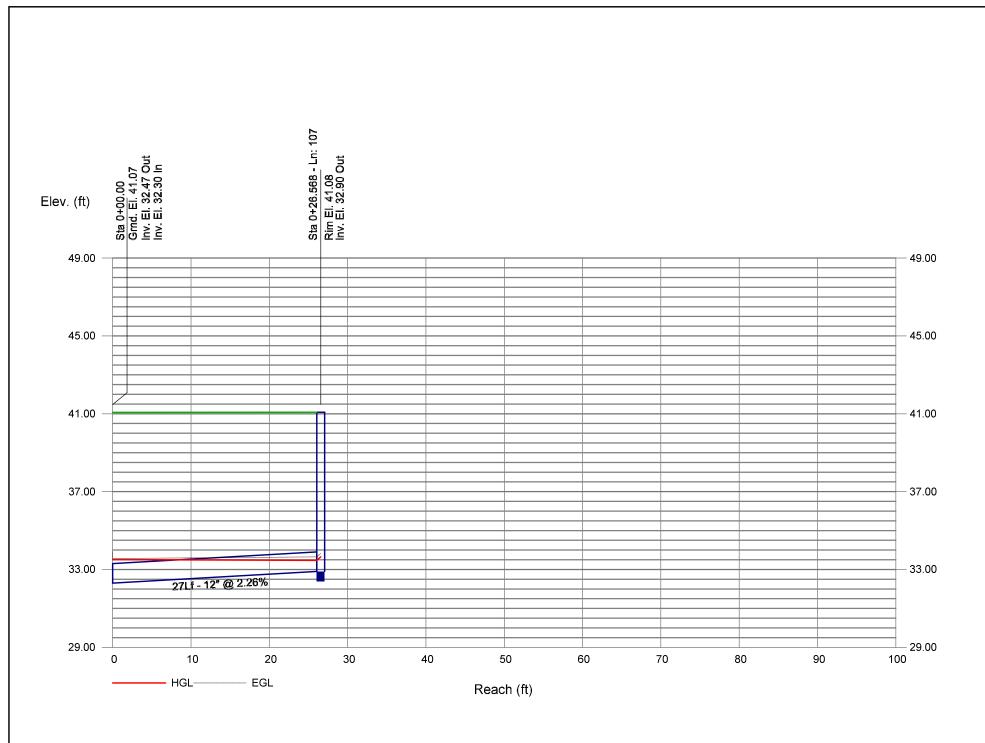


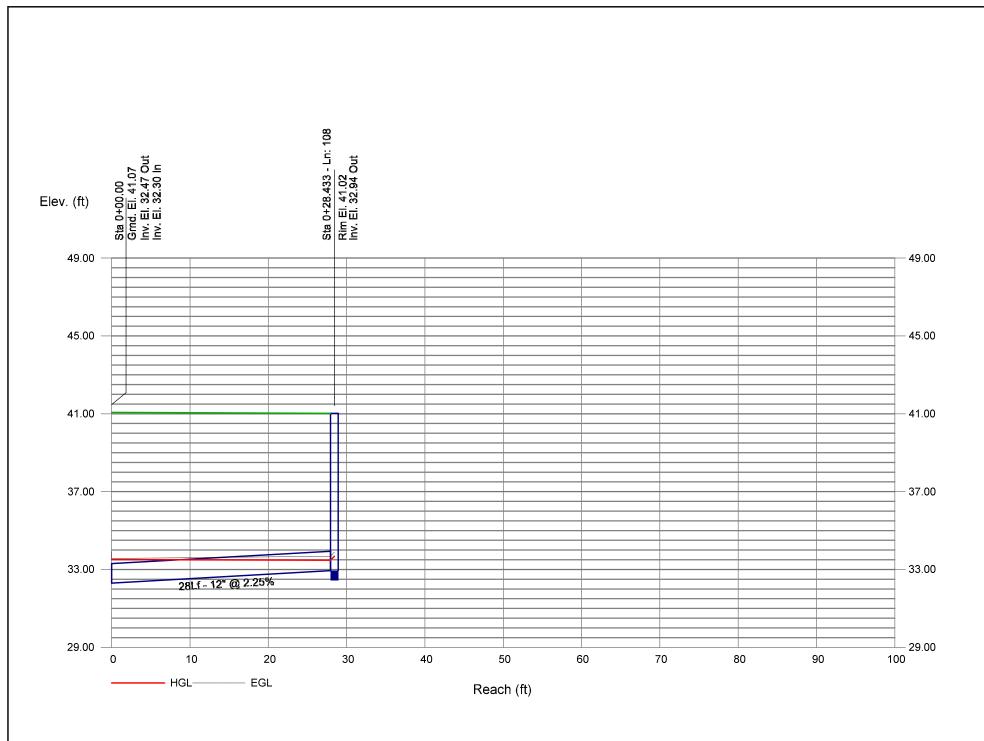


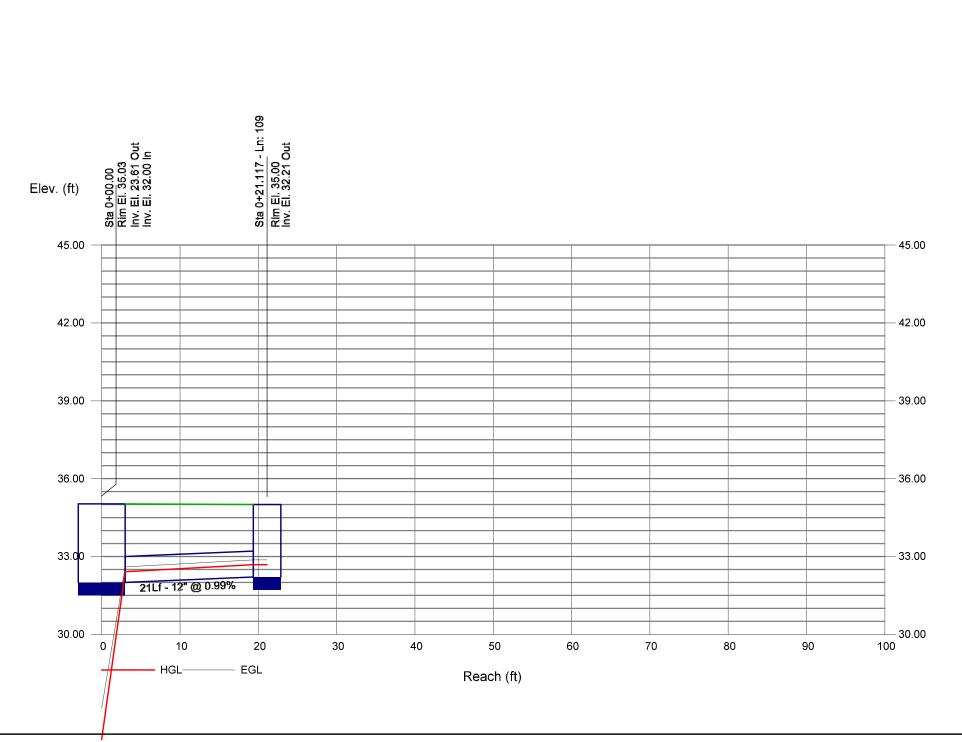


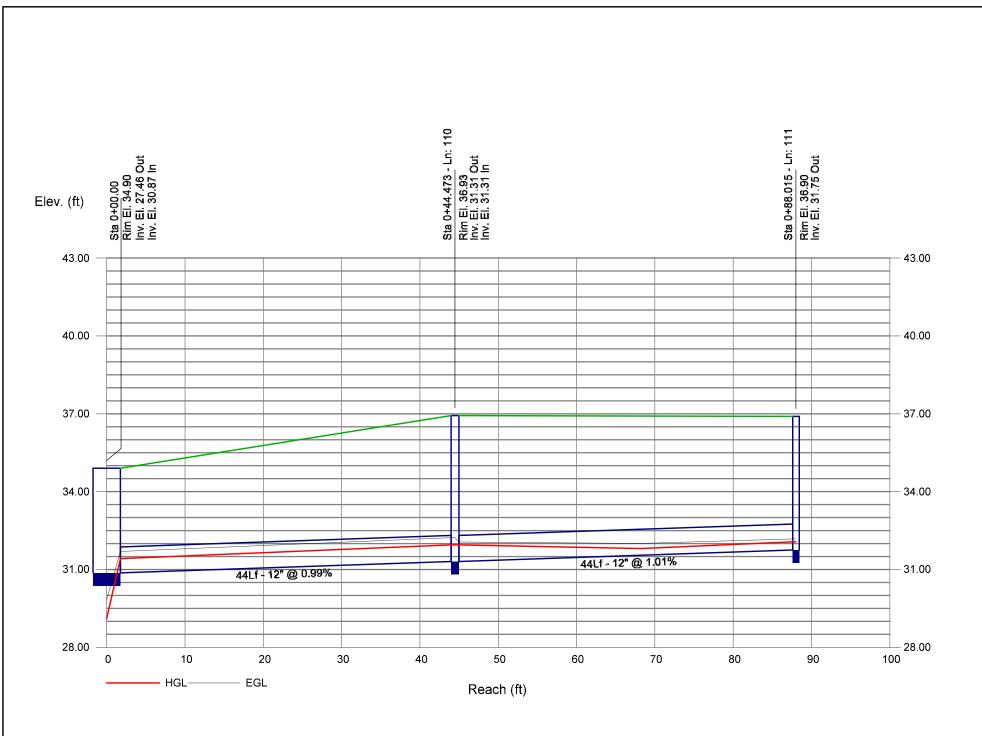


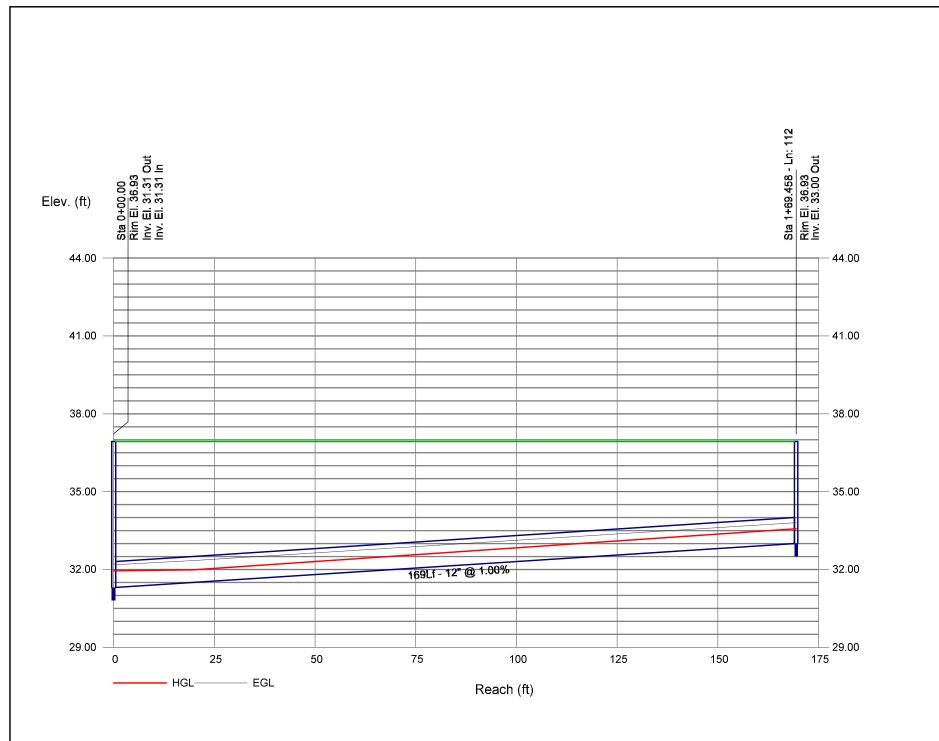


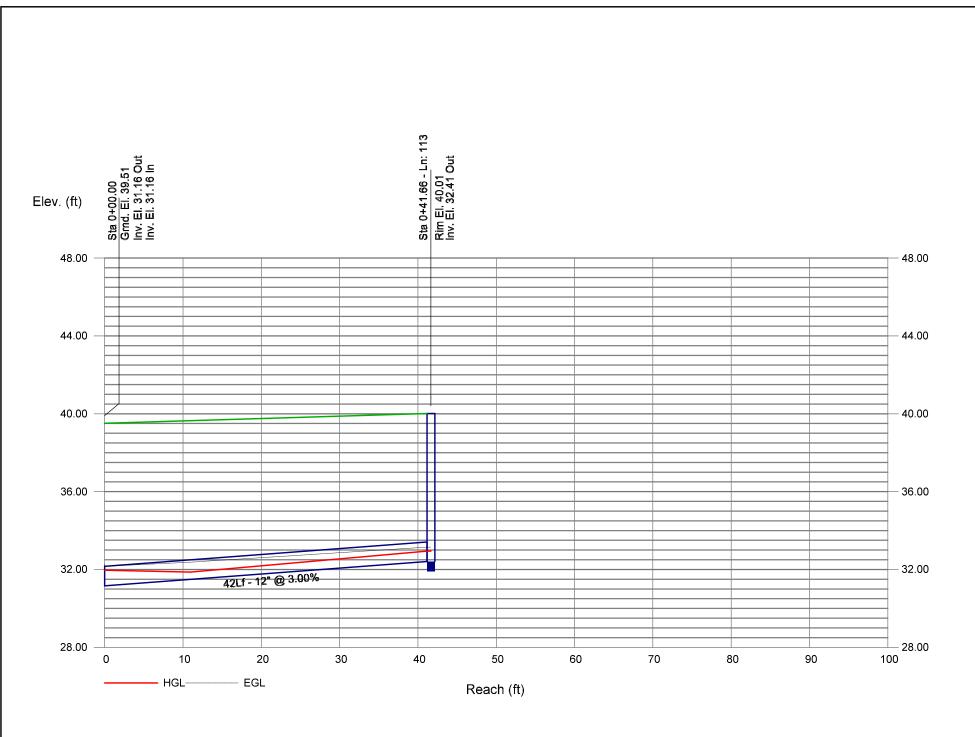


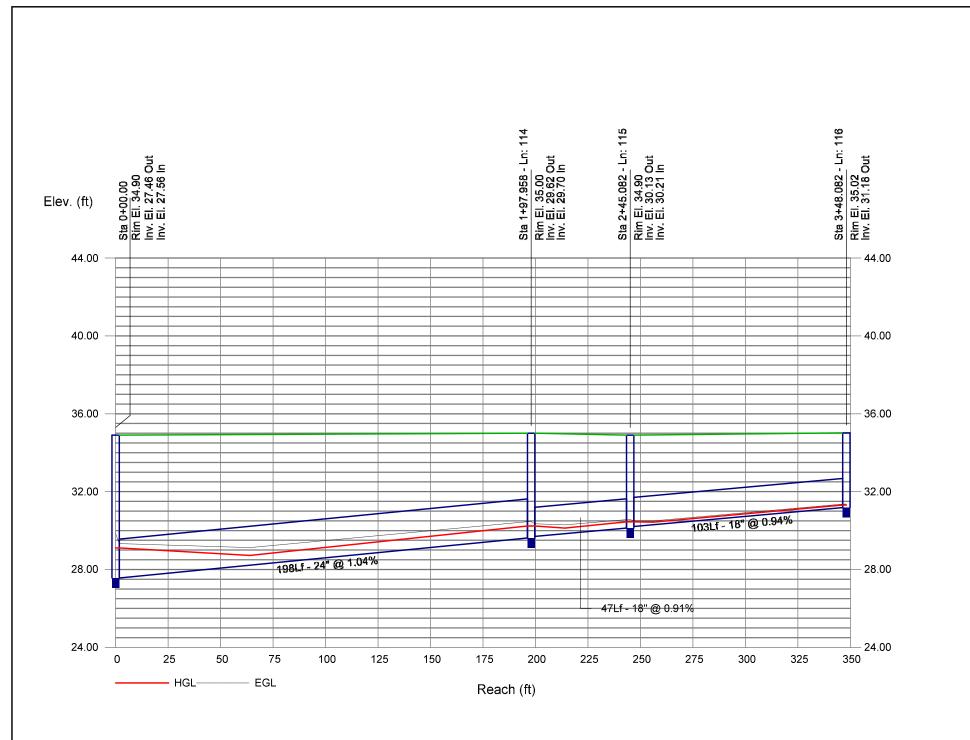


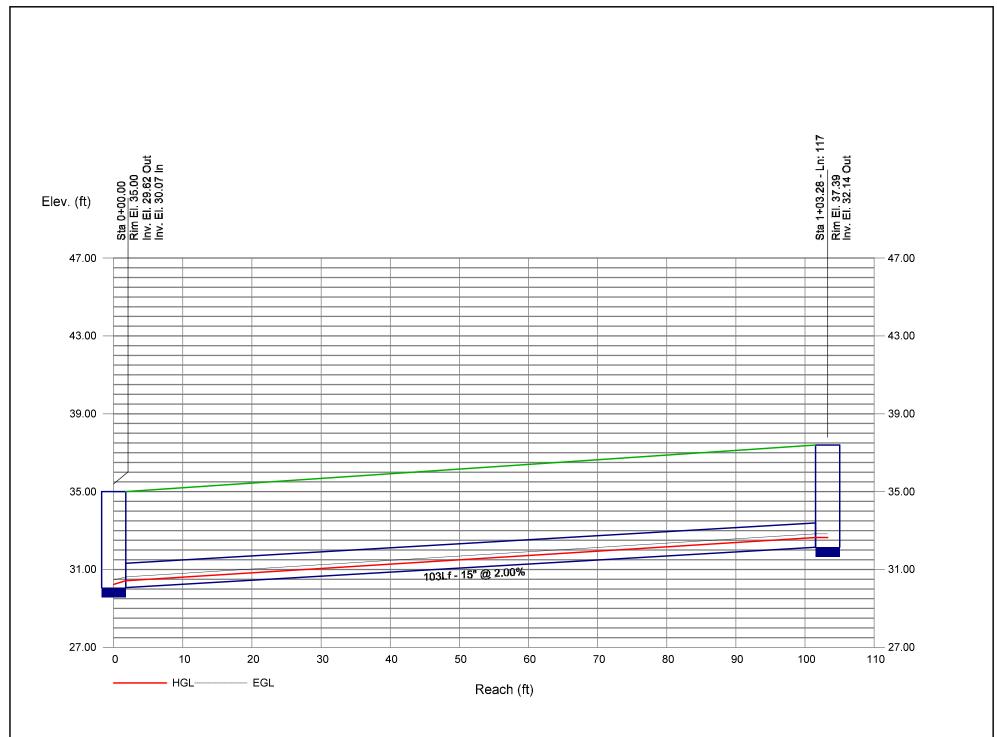


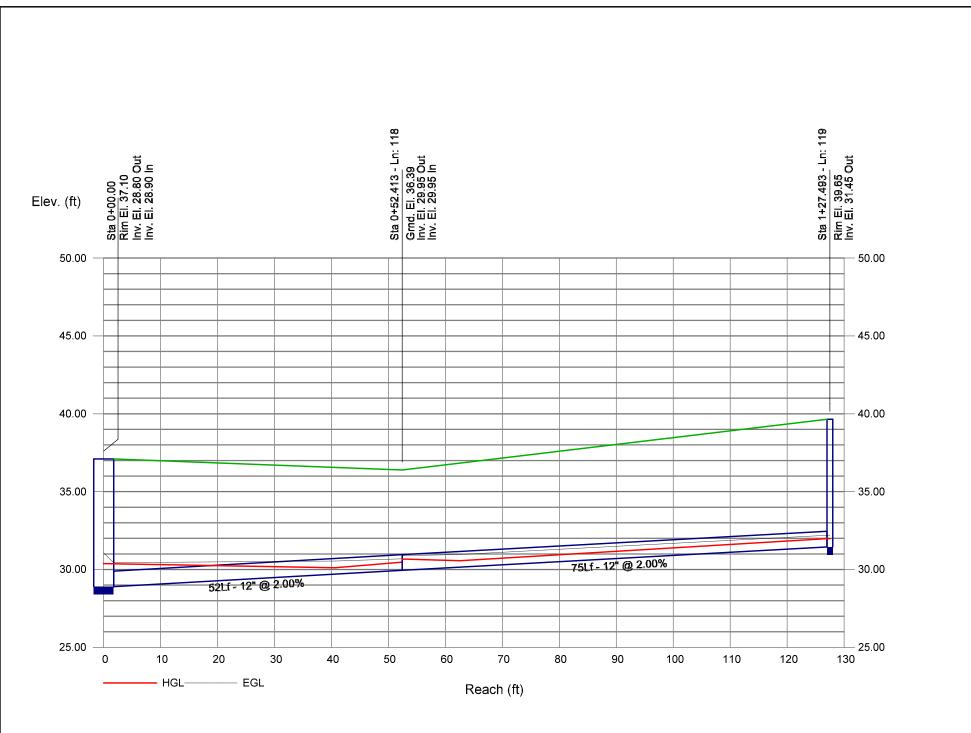


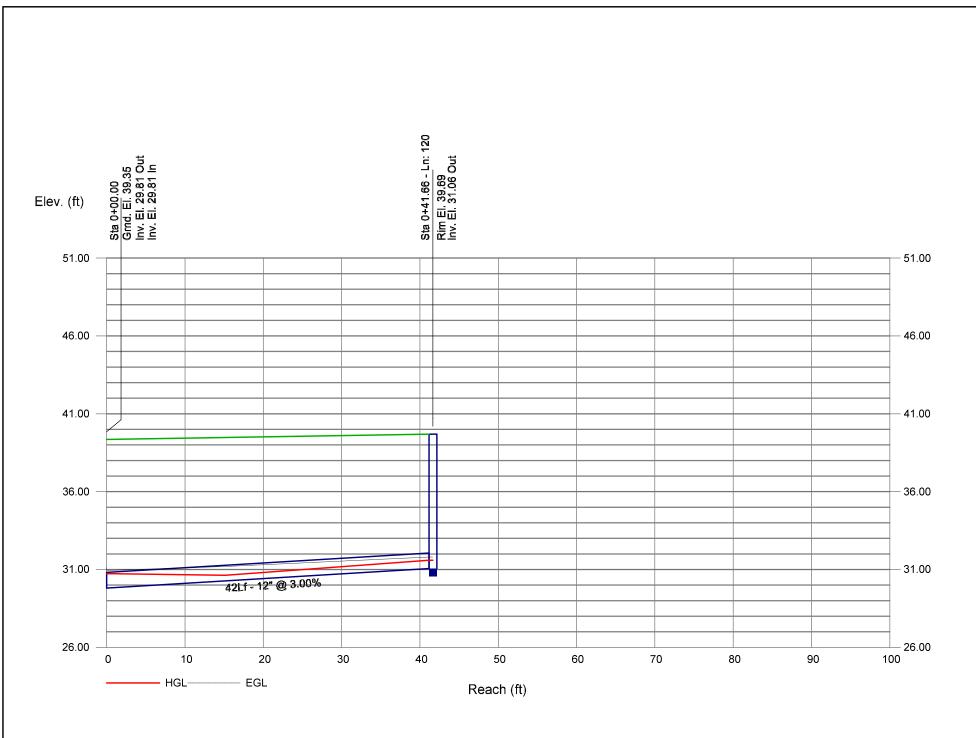


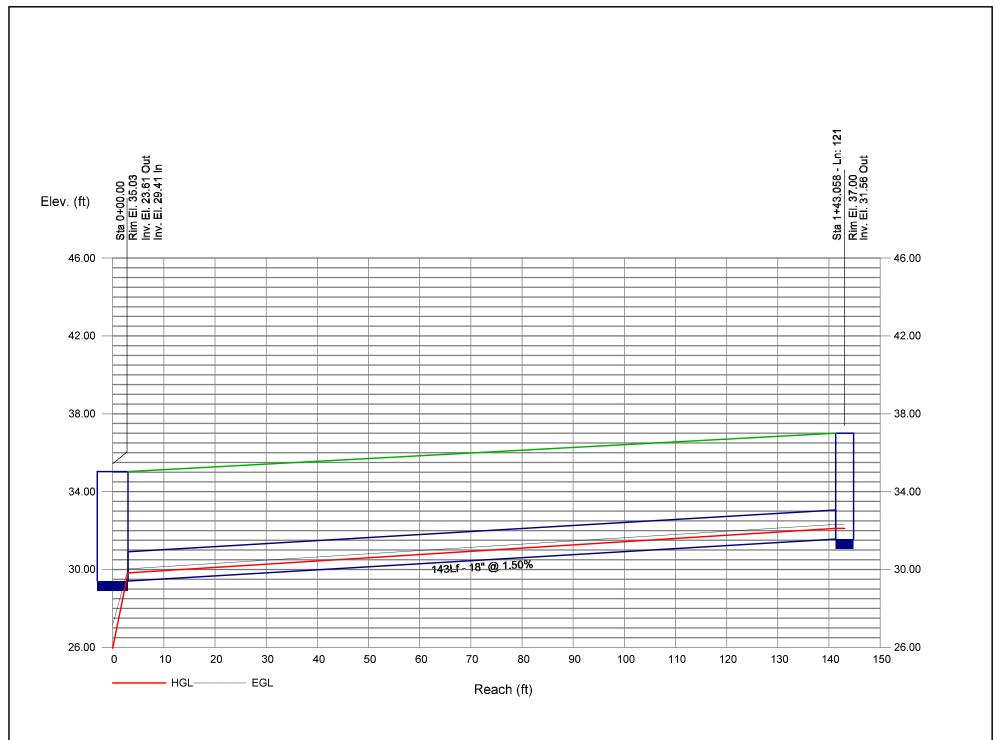


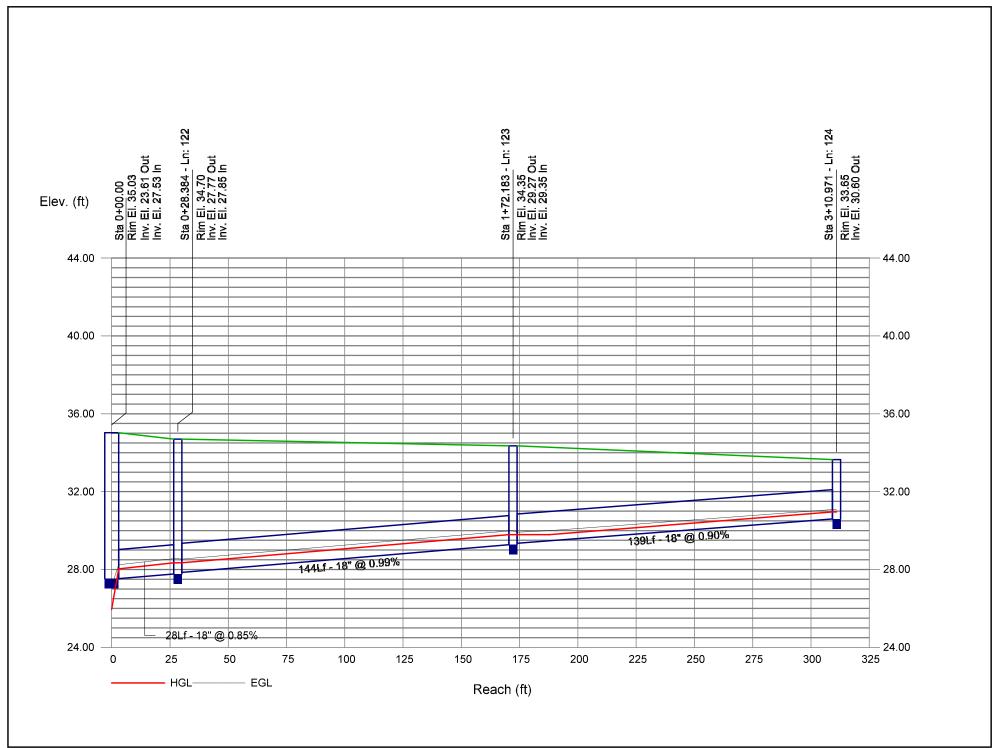


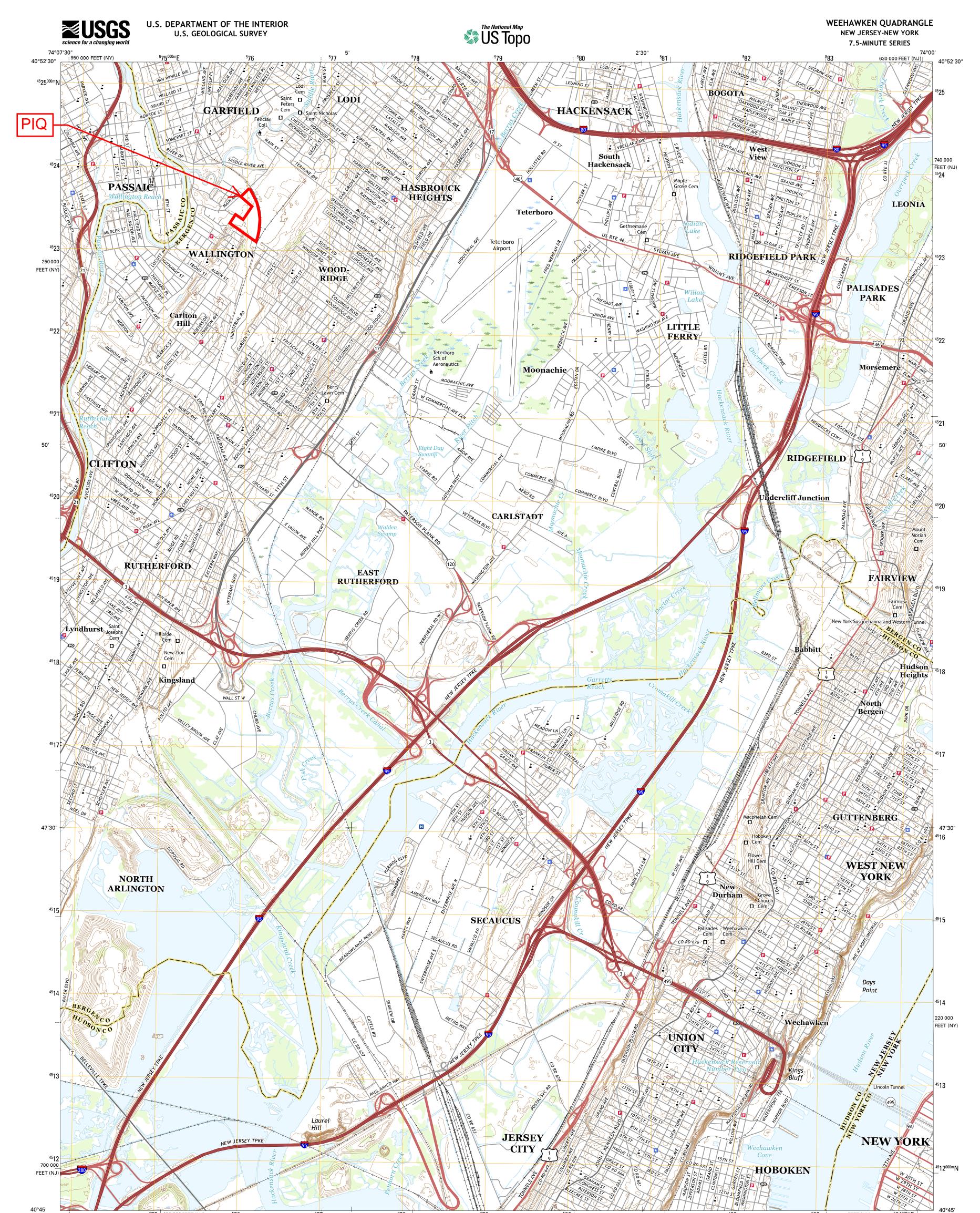






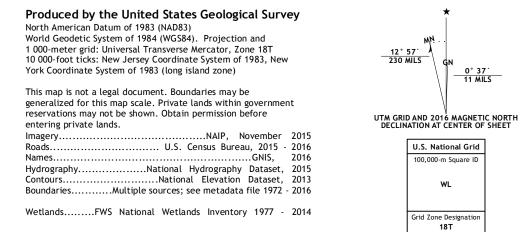






40°45'

74°07'30"

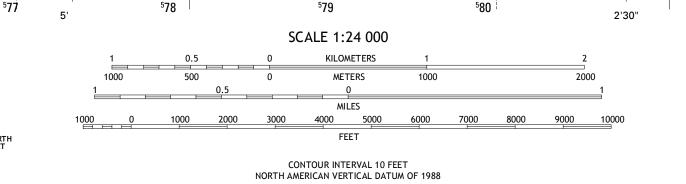


<sup>5</sup>75 600 000 FEET (NJ)

576

0° 37′ 11 MILS

WL



This map was produced to conform with the National Geospatial Program US Topo Product Standard, 2011. A metadata file associated with this product is draft version 0.6.19



<sup>5</sup>82

<sup>5</sup>83 980 000 FEET (NY)

ROAD CLASSIFICATION

584000mE

State Route

Local Connector \_\_\_\_\_

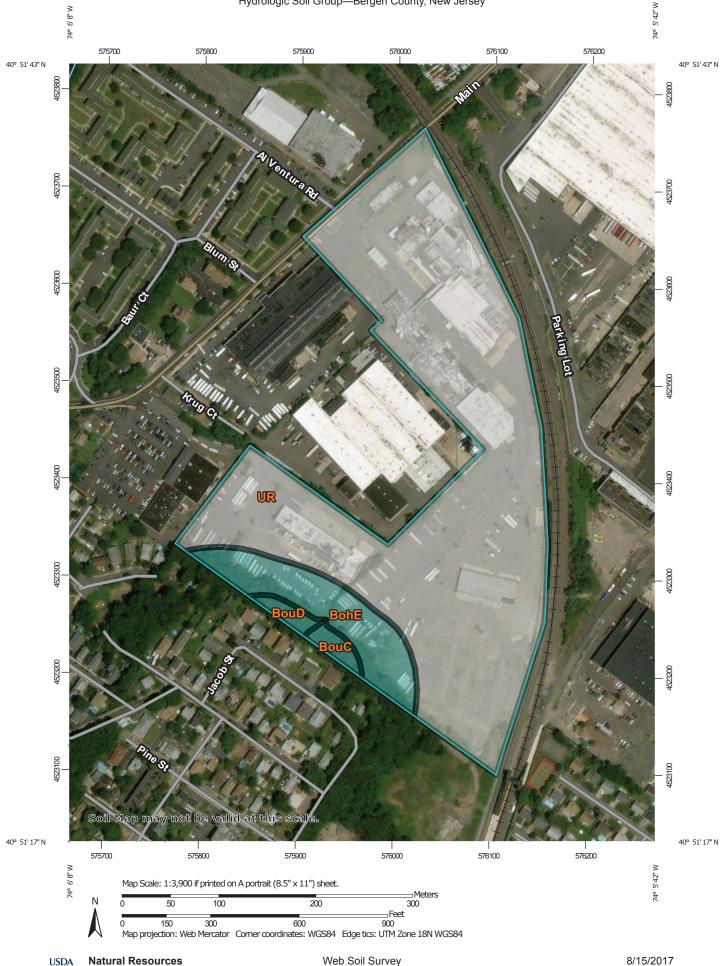
Local Road

4WD

74°00'



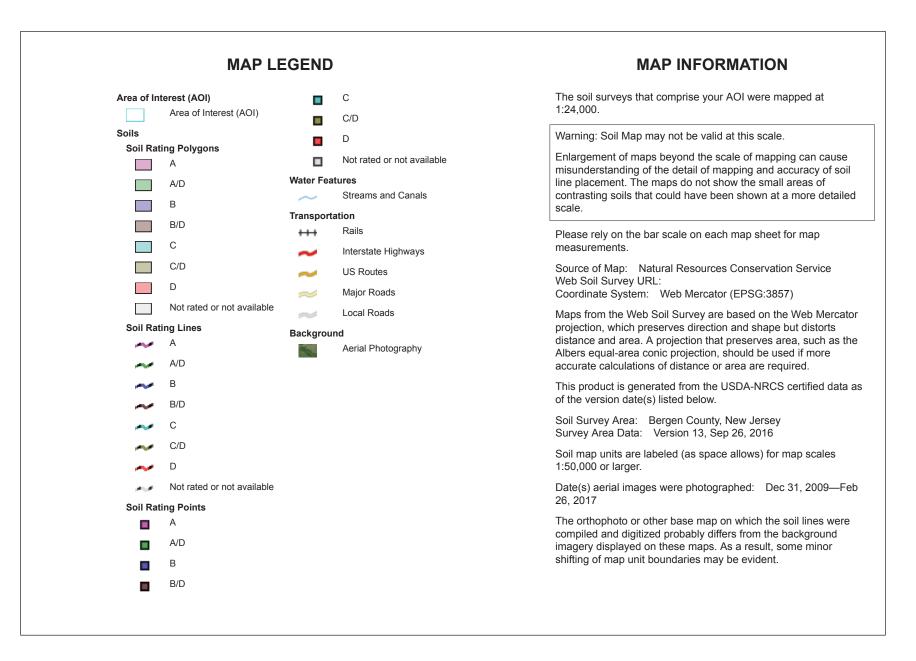
Hydrologic Soil Group—Bergen County, New Jersey



USDA

**Conservation Service** 

Web Soil Survey National Cooperative Soil Survey



# Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Bergen County, New Jersey (NJ003)										
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI						
BohE	Boonton moderately well drained gravelly loam, 25 to 45 percent slopes	C	2.8	10.5%						
BouC	Boonton-Urban land complex, 8 to 15 percent slopes	С	0.3	1.2%						
BouD	Boonton-Urban land complex, 15 to 25 percent slopes	С	0.3	1.1%						
UR	Urban land		23.5	87.1%						
Totals for Area of Intere	est	27.0	100.0%							



#### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

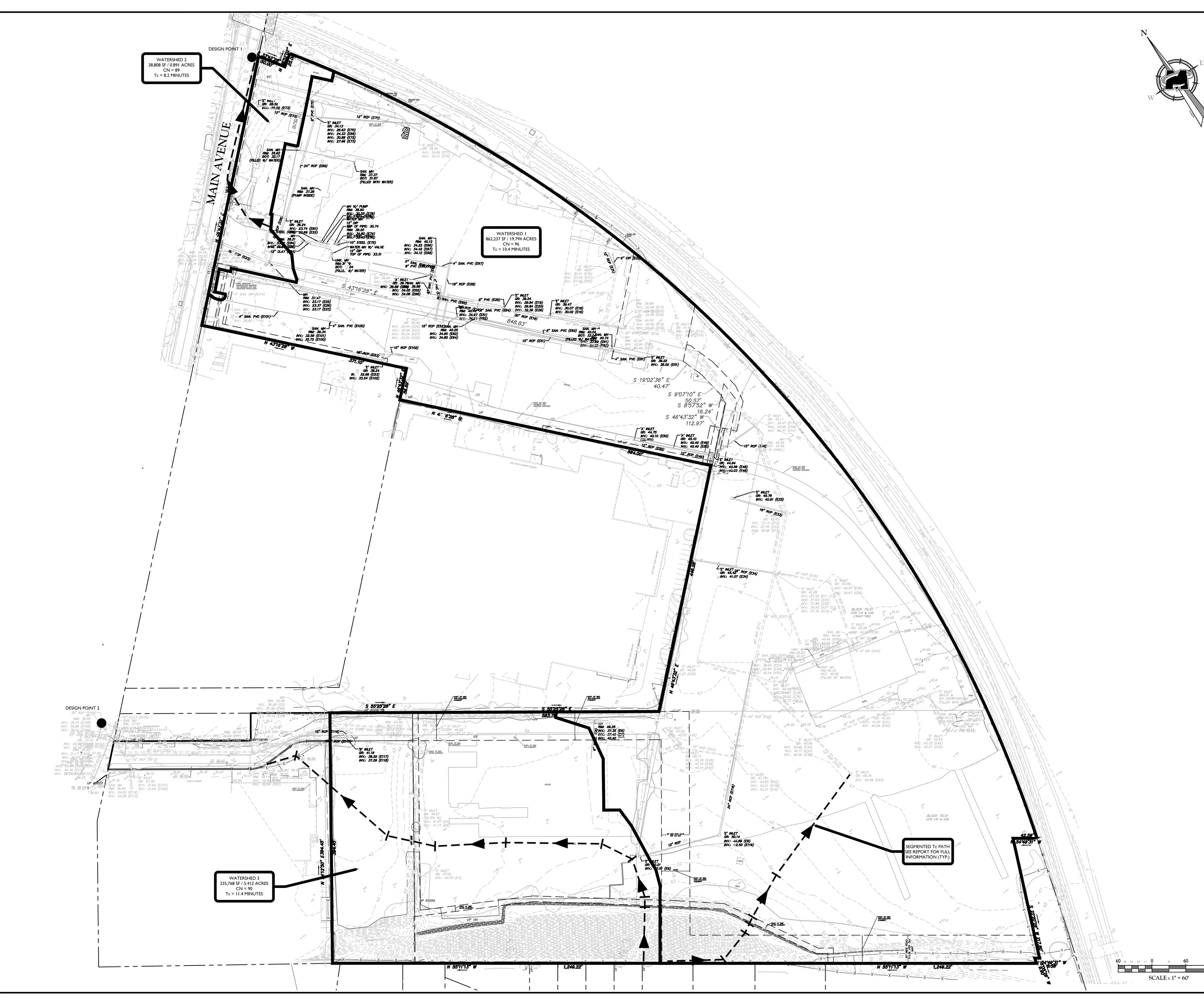
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

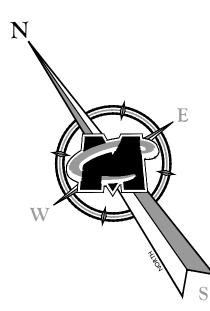
If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

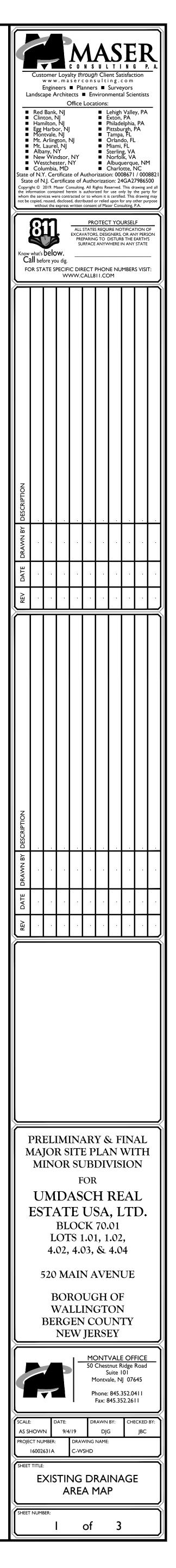
#### **Rating Options**

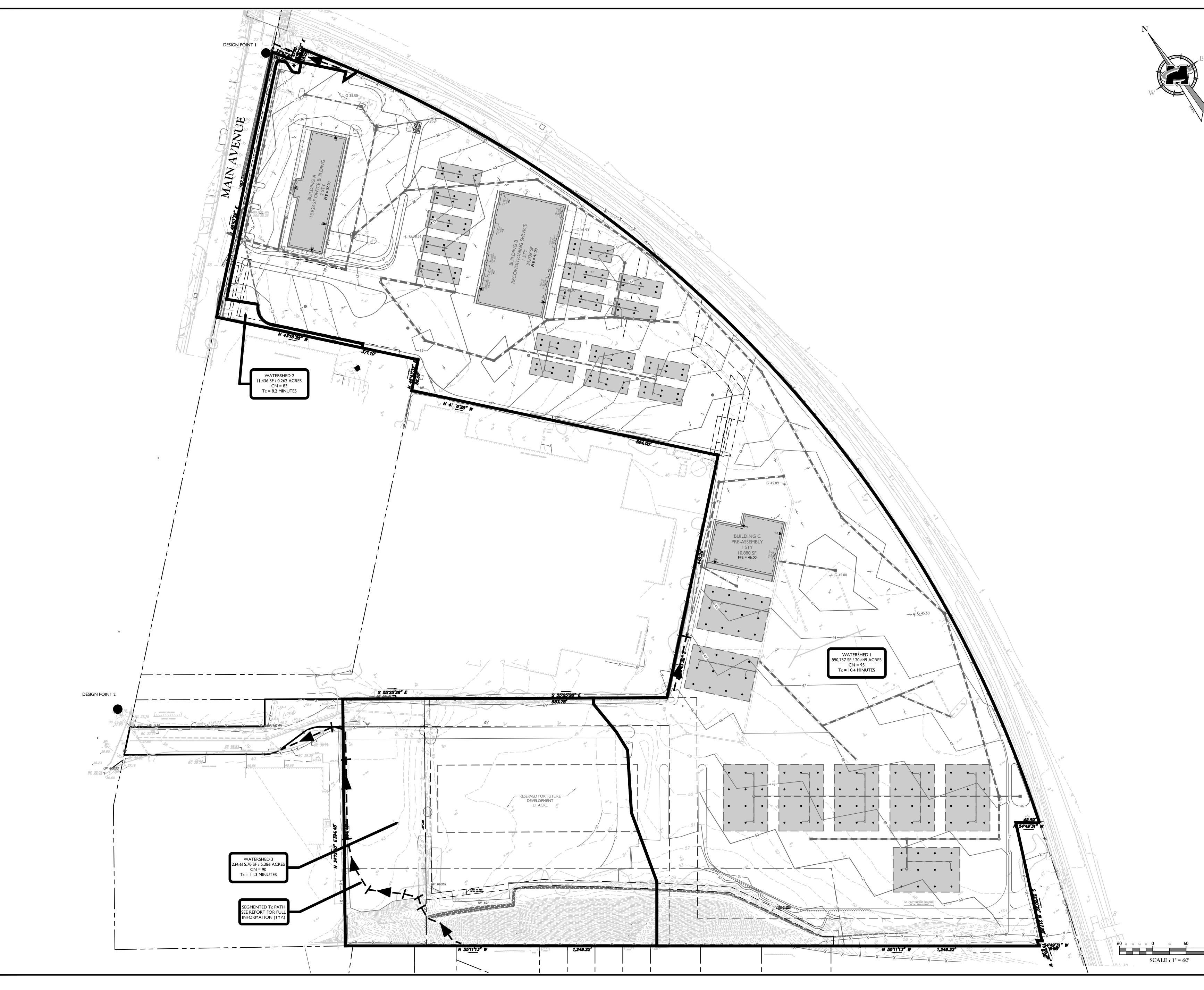
Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

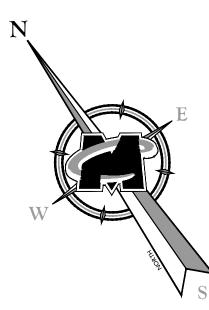


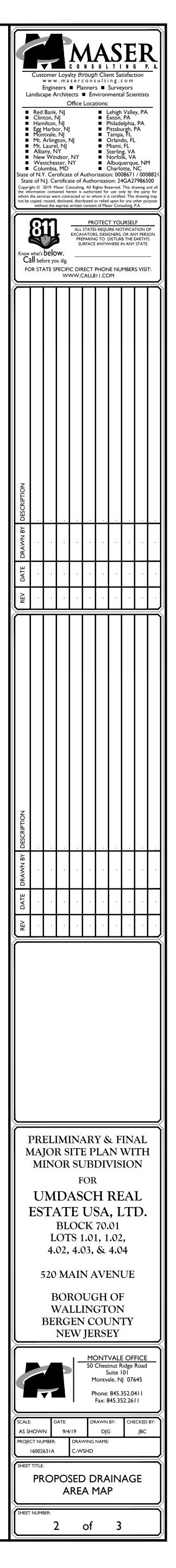


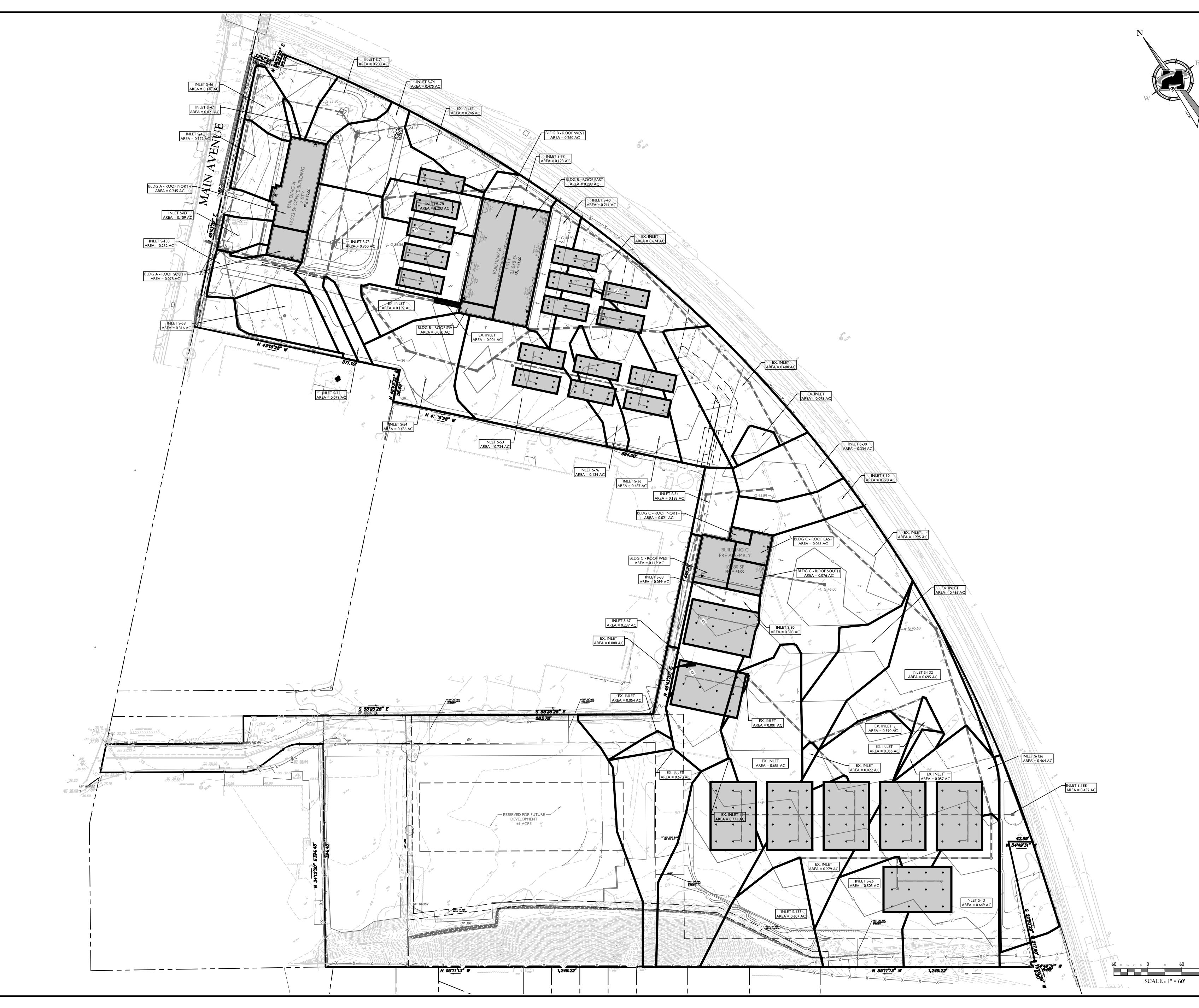


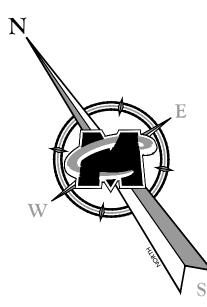












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Landscape Architects Environmental Scientists Office Locations: Red Bank, NJ Lehigh Valley, PA Clinton, NJ Exton, PA Hamilton, NJ Philadelphia, PA Egg Harbor, NJ Pittsburgh, PA Montvale, NJ Tampa, FL Mt. Arlington, NJ Orlando, FL Mt. Laurel, NJ Miami, FL Albany, NY Sterling, VA New Windsor, NY Norfolk, VA Westchester, NY Albuquerque, NM Columbia, MD Charlotte, NC State of N.Y. Certificate of Authorization: 2008671 / 0008821 State of N.J. Certificate of Authorization: 24GA27986500 Copyright 2019. Maser Consulting. All Rights Reserved. This drawing and all the information contained herein is authorized for use only by the party for whom the services were contracted or to whom it is certified. This drawing and all the information contained herein is authorized for use only by the party for whom the services were contracted or to whom it is certified. This drawing and the information contained herein is authorized for use only by the party for whom the services were contracted or to whom it is certified. This drawing may not be copied, reused, disclosed, distributed or relied upon for any other purpose without the express written consent of Maser Consulting. PLA.										
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DRAWN BY DESCRIPTION										
REV DATE DRAWN BY		•		· ·	· ·	· ·	· ·			·
EV DATE DRAWN BY DESCRIPTION										
PRELIMINARY & FINAL MAJOR SITE PLAN WITH MINOR SUBDIVISION FOR UMDASCH REAL ESTATE USA, LTD. BLOCK 70.01 LOTS 1.01, 1.02, 4.02, 4.03, & 4.04 520 MAIN AVENUE BOROUGH OF WALLINGTON BERGEN COUNTY NEW JERSEY										
MONTVALE OFFICE 50 Chestnut Ridge Road Suite 101 Montvale, NJ 07645 Phone: 845.352.0411 Fax: 845.352.2611 SCALE: DATE: DRAWN BY: CHECKED BY:										
AS SHOWN 9/4/19 DJG JBC PROJECT NUMBER: DRAWING NAME: 16002631A C-WSHD SHEET TITLE: INLET DRAINAGE AREA MAP										
SHEET NUMBER: 3 of 3										